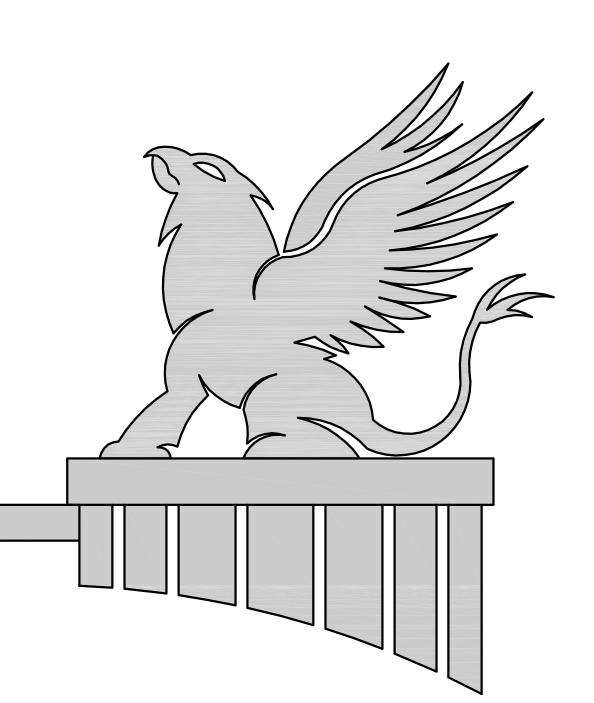


NELSEN RESIDENCE

13222 3RD STREET EAST, MADEIRA BEACH, PINELLAS



PROJECT DETAILS

CODE:

2023 FLORIDA BUILDING CODES-8TH EDITION,

NEC 2020

OCCUPANCY CLASSIFICATION:

RESIDENTIAL, GROUP R3 - SINGLE FAMILY

BUILDING TYPE:

TYPE V-B.

NUMBER OF STORIES:

2 STORY

BUILDING AREA (O.A.):

3128 SQFT

BATHROOMS: **IMPACT ZONE:** 2 TOTAL

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY STEVE GORDILLO, PE USING A DIGITAL SIGNATURE.

PRINTED COPIES OF THIS DOCUMENT ARE NOT **CONSIDERED SIGNED AND** SEALED AND THE SHA **AUTHENTICATION CODE MUST BE VERFIED ON ANY ELECTRONIC COPIES**

A. THE GENERAL CONTRACTOR SHALL VERIFY ALL DESIGN ELEMENTS, CONDITIONS, DIMENSIONS, AND NOTES FOR ACCURACY, SUITABILITY AND CODE COMPLIANCE. THE GENERAL CONTRACTOR SHALL NOTIFY MORGANCASTLE STUDIO, INC. IN WRITING WITH ANY ADJUSTMENTS NEEDED PRIOR TO PLACING ORDERS FOR MATERIALS

MADE PRIOR TO PLACING ORDERS FOR MATERIALS OR BEGINNING CONSTRUCTION

D. DESIGNS AND DRAWINGS PRODUCED BY MORGANCASTLE STUDIO, INC. ARE THE INTELLECTUAL PROPERTY CURTIS R. MORGAN AND ARE PROTECTED UNDER U.S. COPYRIGHT LAW. ANY REPRODUCTION OR UNAUTHORIZED USE IS PROHIBITED WITHOUT EXPRESS WRITTEN PERMISSION. THIS CONSTRUCTION DOCUMENT IS FOR THE CONSTRUCTION OF ONE STRUCTURE AT THE ADDRESS INDICATED AND MAY NOT BE USED FOR ADDITIONAL SITES.

FINALS FOR PERMIT - 11-8-24

STRUCTURAL ENGINEER

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BUILDER

Omar Abbas

Abbas Development, LLC Certified General Contractor

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abbasdevelopmentfl@gmail.com

DESIGNER

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morgancastlestudio@gmail.com

SHEET INDEX

- 1. GROUND FLOOR 2. 2ND FLOOR 3. ELEVATIONS
- 4. ELEVATIONS 5. ELECTRICAL - GROUND
- 6. ELECTRICAL 2ND FLOOR 7. FOUNDATION 8. FRAMING
- 9. FRAMING 10. DETAILS/SECTIONS S1-S6: ENGINEERING DETAILS

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FLOOD NOTES

FLOOD ZONE: AE-10, 4' FREEBOARD - PANEL 12103C0191H - 8-24-21 - 8-24-2021

NON-CONVERSION AGREEMENT TO BE COMPLETED BY HOMEOWNER REQUIRED BEFORE ISSUANCE OF PERMIT FOR ENCLOSURE BUILT BELOW BFE.

UNDER CONSTRUCTION ELEVATION CERTIFICATE REQUIRED PRIOR TO VERTICAL CONSTRUCTION.

FINISHED CONSTRUCTION ELEVATION CERTIFICATE REQUIRED PRIOR TO FINAL INSPECTION.

FLOOD OPENINGS REQUIRED FOR CONSTRUCTION BELOW BFE MUST MEET ALL REQUIREMENTS OF ASCE, FBC 2023, 8TH ED, 44 CFR & FEMA TB 1-20.

ENCLOSURE BELOW BFE LIMITED TO ONLY PARKING, UNFINISHED STORAGE OR BUILDING ACCESS SECTION 60.3, 44 CFR & TB 1-20.

ALL UTILITIES SERVICING BUILDING MUST BE ELEVATED ABOVE FLOOD. PROTECTION LEVEL (BFE PLUS 1 FT) OR DRY FLOODPROOFED TO THE FLOOD PROTECTION LEVEL. (FEMA P-348).

GENERAL NOTES

- ALL EXTERIOR FRAME WALLS AND INTERIOR BEARING WALLS TO BE 2X6 WOOD STUDS AT 16"
- O.C. SEE "S" SHEETS FOR DETAILS.
- STRUCTURAL NOTES FOR LINTELS, HEADERS ,BEAMS, COLUMNS AND UPLIFT CONNECTION CALL-OUTS ARE ON THE STRUCTURAL FRAMING & TRUSS LAYOUT SHEET.
- OPENINGS BETWEEN THE GARAGE AND RESIDENCE SHALL BE EQUIPPED WITH A SELF
- CLOSING, 20 MIN FIRE RATED DOOR PER FBCR 302.5.1 ALL PLUMBING, ELECTRICAL, AND MECHANICAL ROUGH-INS MUST BE COMPLETE, INSPECTED,
- AND APPROVED BEFORE REQUESTING THE FRAMING INSPECTION. FBC,R R109.3 PROVIDE ONE 29"W CLEAR OPENING TO A BATHROOM ON THE FIRST FOR FOR HANDICAP
- ACCESS. FBC-R R320.
- ALL GLASS IN HAZARDOUS LOCATIONS PER R308.4 TO BE TEMPERED GLASS. PROVIDE A MINIMUM OF ONE 36" SIDE HINGED EXIT DOOR ON THE FIRST FLOOR PER R311.2.
- ALL NON WALK-IN CLOSET CEILINGS TO BE A MAXIMUM OF 8' HIGH. PLANS USE MODULAR DIMENSIONS. 2X4 FRAME WALLS ARE ASSUMED 4", PLUMBING WALLS 6" AND CMU WALLS ARE 8" UNLESS OTHERWISE NOTED. ADD OR SUBTRACT TO GET CENTER
- 0. CONFIRM MASONRY AND FRAME EXTERIOR OPENING REQUIREMENTS WITH WINDOW/DOOR
- SUPPLIER BEFORE CONSTRUCTION. UNIT REQUIREMENTS TAKE PRECENDENCE. . ALL EXTERIOR FRAME WALLS HAVE R-13 BATT INSULATION AND VAPOR BARRIER PER FBC-RE
- TABLE R402.1.2. 12. ALL EXTERIOR CEILINGS (I.E. PORCHES, ENTRY) HAVE $\frac{7}{8}$ "T MIN. STUCCO ON WIRE LATHE OVER 30LB FELT OVER BUILDING WRAP. FINISH MATERIALS TO COMPLY WITH R703, R703.7
- 13. PROVIDE CONCRETE STOOPS FOR EXTERIOR DOORS TO GRASSED AREAS. 14. ALL SHELVING TO BE VINYL COATED WIRE SHELVING.
- 15. G.C. AND SUBS/SUPPLIERS TO VERIFY ALL CONDITIONS PRIOR TO COMMENCEMENT OF
- 16. CLOSET ORGANIZERS BUILT-IN BY OWNER.

WALL LEGEND

2X NON-BEARING WALL:

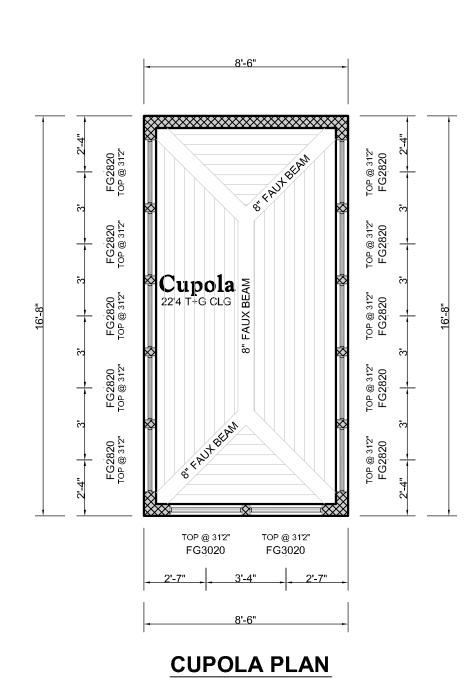
2X BEARING WALL W/ UPLIFT: 8" CMU EXTERIOR BEARING WALL:

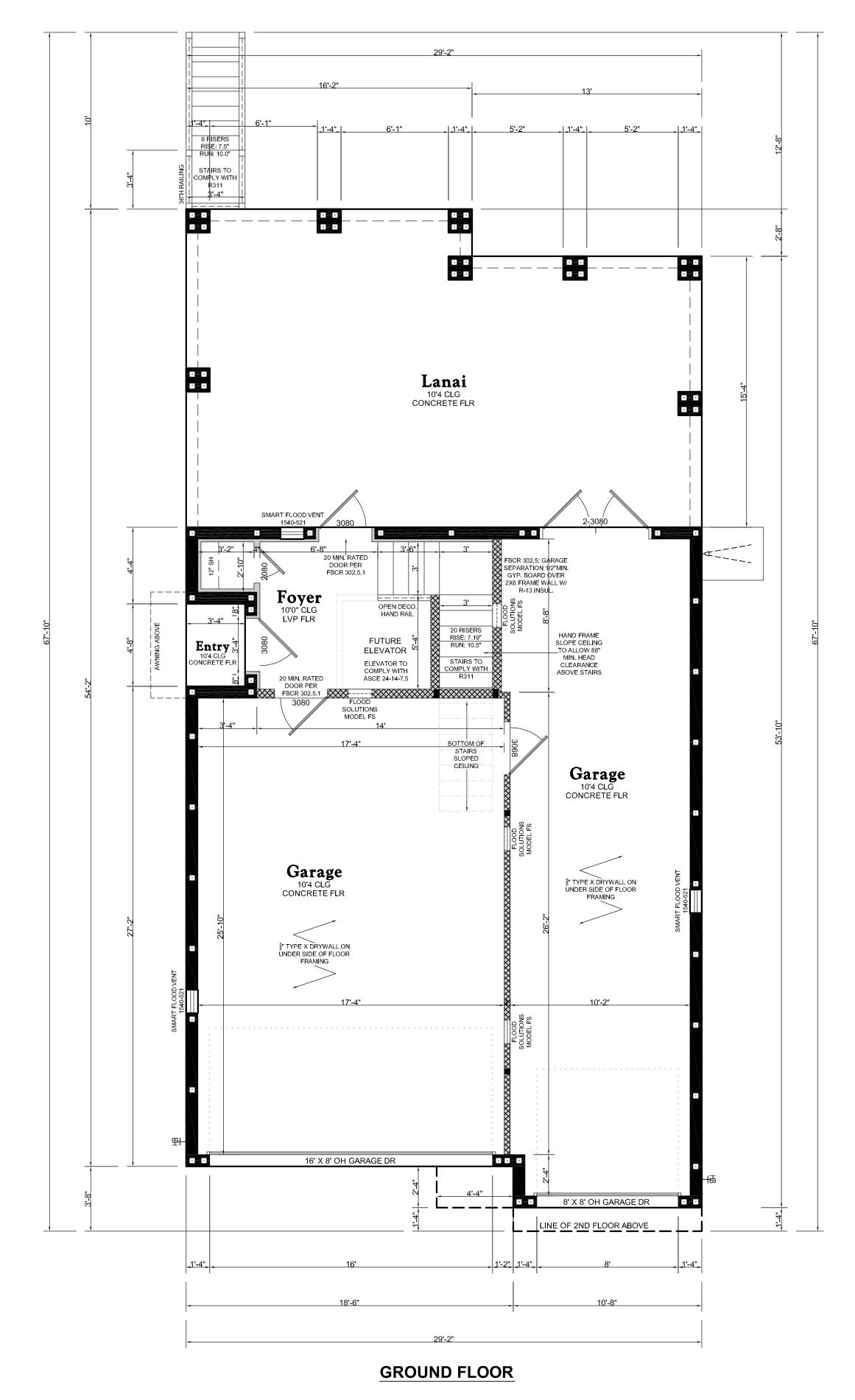
(3) SMART VENTS VENT MODEL 1540-52, PA: FL 5822.4 RATED AT 400 SF EACH 1064 SF REQUIRED 1200 SF SUPPLIED

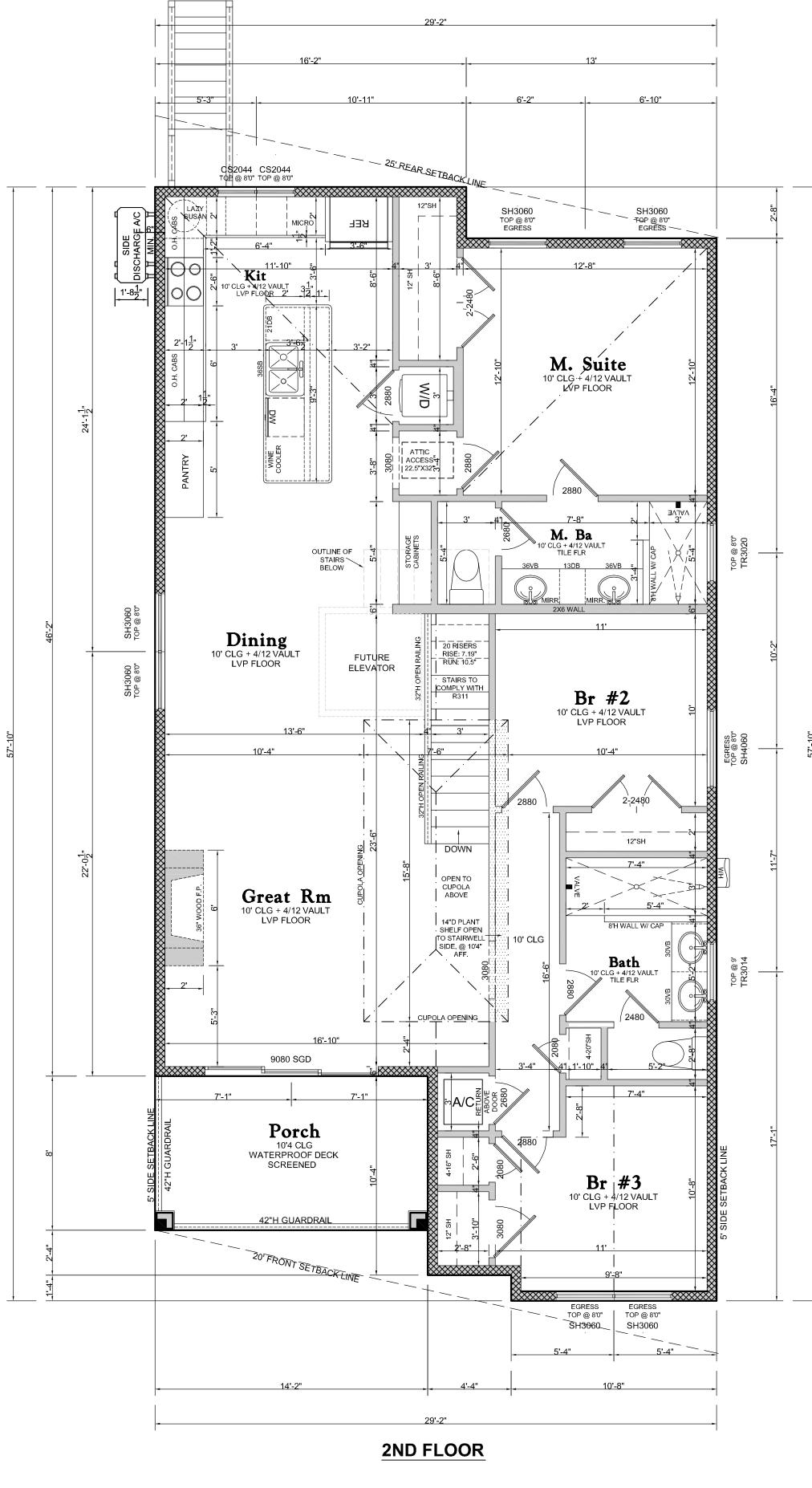
FLOOD VENT CALCS

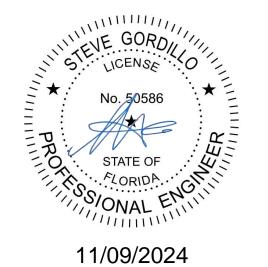
DOOR AND WINDOW LABELS

- A SH3060 WINDOW IS A SINGLE-HUNG STYLE 3'-0" WIDE X 6'-0" HIGH. A TR8014 IS A TRANSOM 8'-0" WIDE X 1'-4" HIGH.
- A 2480 DOOR LABEL IS READ 2'-4" WIDE X 8'-0" HIGH.









AREA CALCULATION LIVING AREA: GARAGE: F. PORCH: 113 LANAI: 490 ENTRY: TOTAL: 3128

FLOOR PLAN

SHEET

FALL PREVENTION

R312.2.1.2: OPERABLE WINDOWS LOCATED LESS THAN 24" ABOVE FINISHED FLOOR AND GREATER THAN 72" ABOVE FINISHED GRADE SHALL BE PROVIDED WITH WINDOW FALL PREVENTION DEVICES THAT COMPLY WITH ASTM F2090

ASCE 24

ALL COMPONENTS BELOW THE DFE WILL BE FLOOD RESISTANT PER ASCE 24 INCLUDING BUILDING, MECHANICAL, ELECTRICAL, PLUMBING, AND GAS TRADES.

EXPANDED FOAM INSULATION

ATTIC VENTILATION - FBC-R SECTION R806 NO VENTING REQUIRED IF ATTIC SPACE ASSEMBLY MEETS FBC-R R806.5 CONDITIONS.

GENERAL NOTES

ALL DIMENSIONS TO BE FIELD VERIFIED.

DIMENSIONS FOR WINDOWS ARE "GENERIC" AND USED FOR DESIGN PURPOSES ONLY.

VERIFY ALL WINDOW OPENINGS WITH WINDOW MANUFACTURER FOR EXACT ROUGH OPENING SIZES

ALL GLAZED OPENINGS SHALL BE IMPACT RESISTANT.

ALL PERIMETER WALLS ARE TO BE CONSIDERED SHEAR WALLS EXCEPT AT DOOR AND WINDOW OPENINGS AND WALL LENGTHS LESS THAN 2'-8". NAILING PATTERN AND SPACING AT SHEATHING FOR SHEAR APPLY TO ALL EXTERIOR FRAME WALLS

FLASHING NOTES

DUE TO CLARITY NOT ALL REQUIRED FLASHING IS INDICATED ON THE DRAWINGS, FLASHING SHALL BE INSTALLED PER FBC 2023 R703.4. CODE SECTION HAS BEEN PROVIDED BELOW AS REFERENCE ONLY

R703.4 FLASHING. APPROVED CORROSION-RESISTANT FLASHING SHALL BE APPLIED SHINGLE-FASHION IN A MANNER TO PREVENT ENTRY OF WATER INTO THE WALL CAVITY OR PENETRATION OF WATER TO THE BUILDING STRUCTURAL FRAMING COMPONENTS. SELF-ADHERED MEMBRANES USED AS FLASHING SHALL COMPLY WITH AAMA 711. ALL EXTERIOR FENESTRATION PRODUCTS SHALL BE SEALED AT THE JUNCTURE WITH THE BUILDING WALL WITH A SEALANT COMPLYING WITH AAMA 800 OR ASTM C920 CLASS 25 GRADE NS OR GREATER FOR PROPER JOINT EXPANSION AND CONTRACTION, ASTM C1281, AAMA 812, OR OTHER APPROVED STANDARD AS APPROPRIATE FOR THE TYPE OF SEALANT. FLUID-APPLIED MEMBRANES USED AS FLASHING IN EXTERIOR WALLS SHALL COMPLY WITH AAMA 714. THE FLASHING SHALL EXTEND TO THE SURFACE OF THE EXTERIOR WALL FINISH. APPROVED CORROSION-RESISTANT FLASHINGS SHALL BE INSTALLED AT THE FOLLOWING LOCATIONS:

1. EXTERIOR WINDOW AND DOOR OPENINGS. FLASHING AT EXTERIOR WINDOW AND DOOR OPENINGS SHALL EXTEND TO THE SURFACE OF THE EXTERIOR WALL FINISH OR TO THE WATER-RESISTIVE BARRIER COMPLYING WITH SECTION 703.2 FOR SUBSEQUENT DRAINAGE. MECHANICALLY ATTACHED FLEXIBLE FLASHINGS SHALL COMPLY WITH AAMA 712. FLASHING AT EXTERIOR WINDOW AND DOOR OPENINGS SHALL BE INSTALLED IN ACCORDANCE WITH ONE OR MORE OF THE FOLLOWING:

1.1. THE FENESTRATION MANUFACTURER'S INSTALLATION AND FLASHING INSTRUCTIONS, OR FOR APPLICATIONS NOT ADDRESSED IN THE FENESTRATION MANUFACTURER'S INSTRUCTIONS, IN ACCORDANCE WITH THE FLASHING MANUFACTURER'S INSTRUCTIONS. WHERE FLASHING INSTRUCTIONS OR DETAILS ARE NOT PROVIDED, PAN FLASHING SHALL BE INSTALLED AT THE SILL OF EXTERIOR WINDOW AND DOOR OPENINGS. PAN FLASHING SHALL BE SEALED OR SLOPED IN SUCH A MANNER AS TO DIRECT WATER TO THE SURFACE OF THE EXTERIOR WALL FINISH OR TO THE WATER-RESISTIVE BARRIER FOR SUBSEQUENT DRAINAGE. OPENINGS USING PAN FLASHING SHALL INCORPORATE FLASHING OR PROTECTION AT THE HEAD

1.2. IN ACCORDANCE WITH THE FLASHING DESIGN OR METHOD OF A REGISTERED DESIGN PROFESSIONAL. 1.3. IN ACCORDANCE WITH OTHER APPROVED

1.4. IN ACCORDANCE WITH FMA/AAMA 100, FMA/AAMA 200, FMA/WDMA 250, FMA/AAMA/ WDMA 300 OR FMA/AAMA/WDMA 400.

2. AT THE INTERSECTION OF CHIMNEYS OR OTHER MASONRY CONSTRUCTION WITH FRAME OR STUCCO WALLS, WITH PROJECTING LIPS ON BOTH SIDES UNDER STUCCO COPINGS.

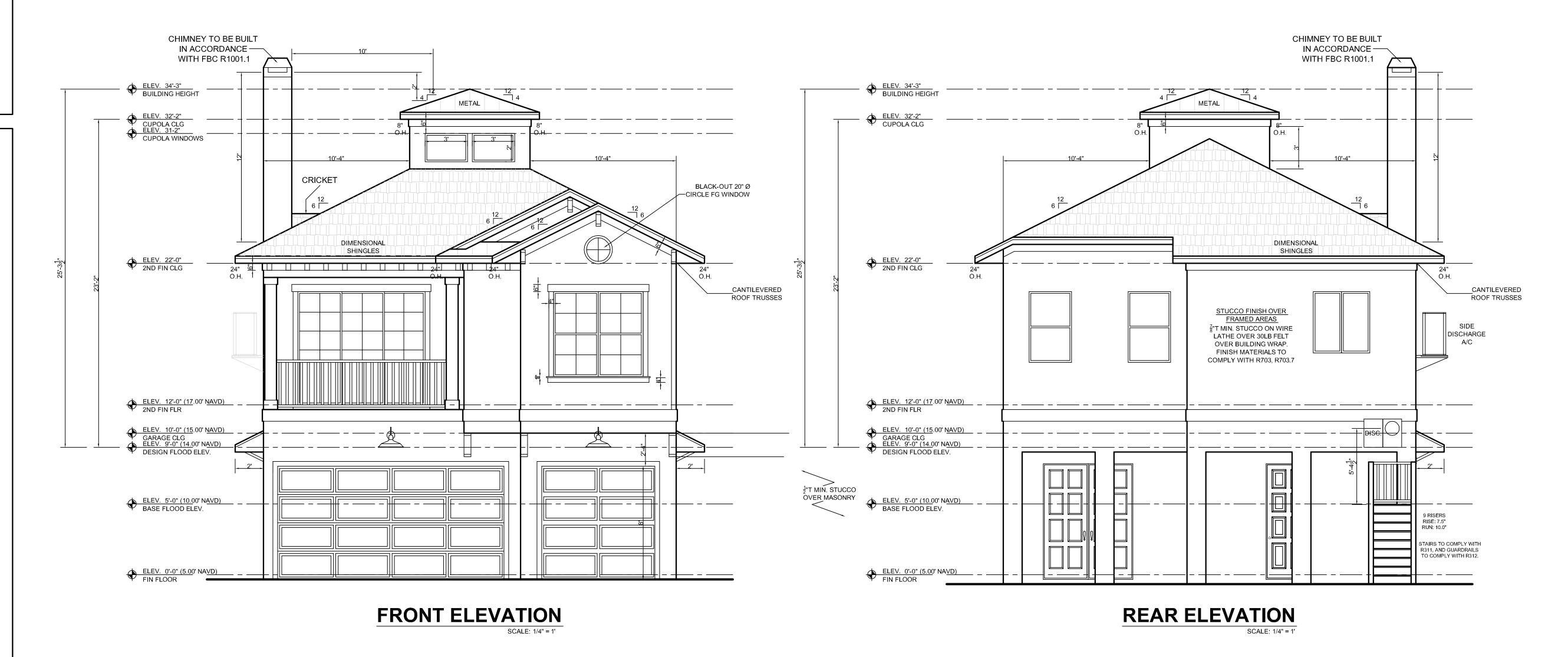
3. UNDER AND AT THE ENDS OF MASONRY, WOOD OR METAL COPINGS AND SILLS.

4. CONTINUOUSLY ABOVE ALL PROJECTING WOOD TRIM. 5. WHERE EXTERIOR PORCHES, DECKS OR STAIRS ATTACH TO A WALL OR FLOOR ASSEMBLY OF

WOOD-FRAME CONSTRUCTION. 6. AT WALL AND ROOF INTERSECTIONS.

7. AT BUILT-IN GUTTERS.

THESE DRAWINGS ARE THE PROPERTY OF CURTIS R. MORGAN. ANY REPRODUCTION OR UNAUTHORIZED USE IS PROHIBITED.



ELEVATIONS SOLE 1/4" OF

SHEET

11/09/2024

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APPLICATIONS NOT ADDRESSED IN THE FENESTRATION
MANUFACTURER'S INSTRUCTIONS, IN ACCORDANCE
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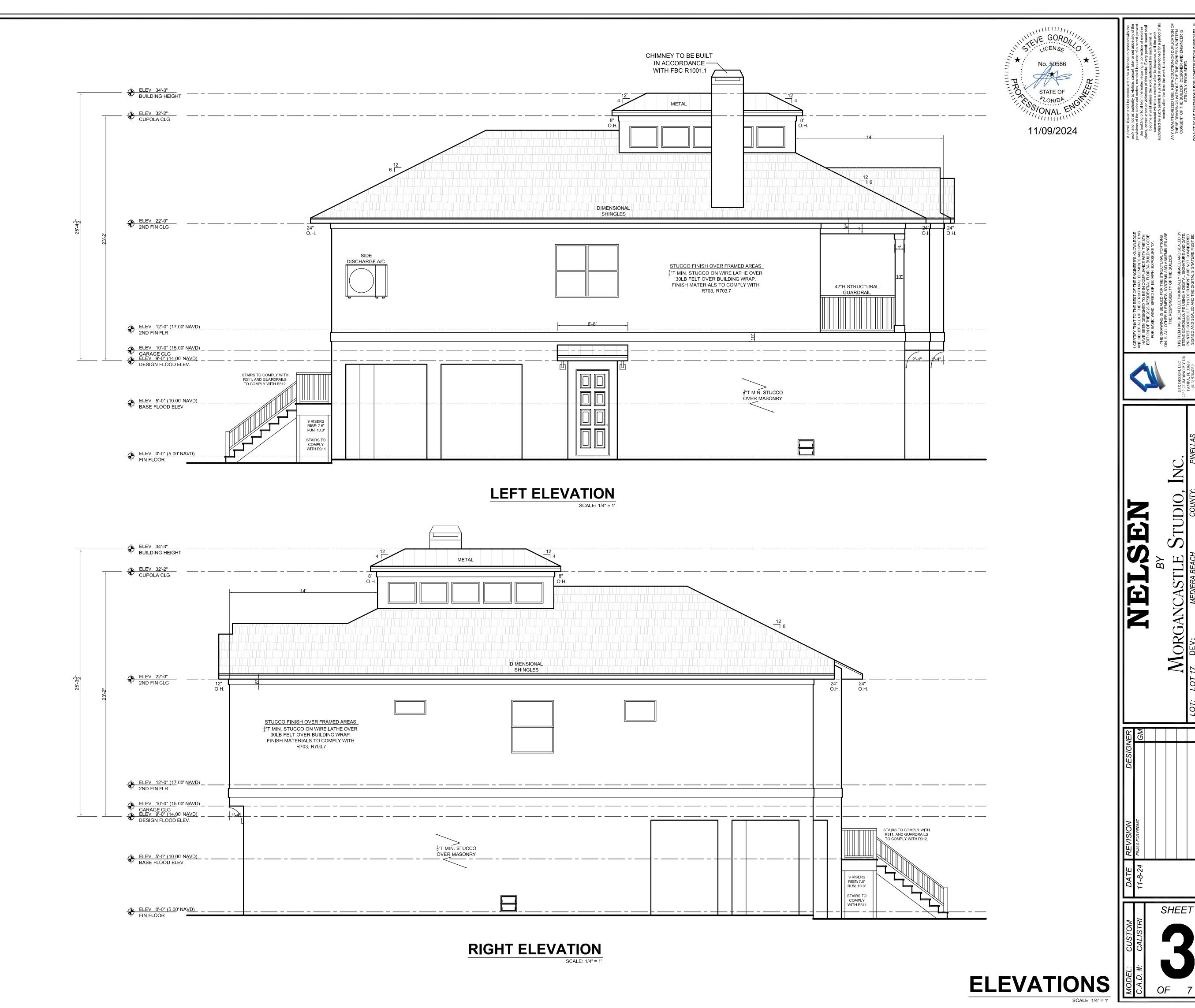
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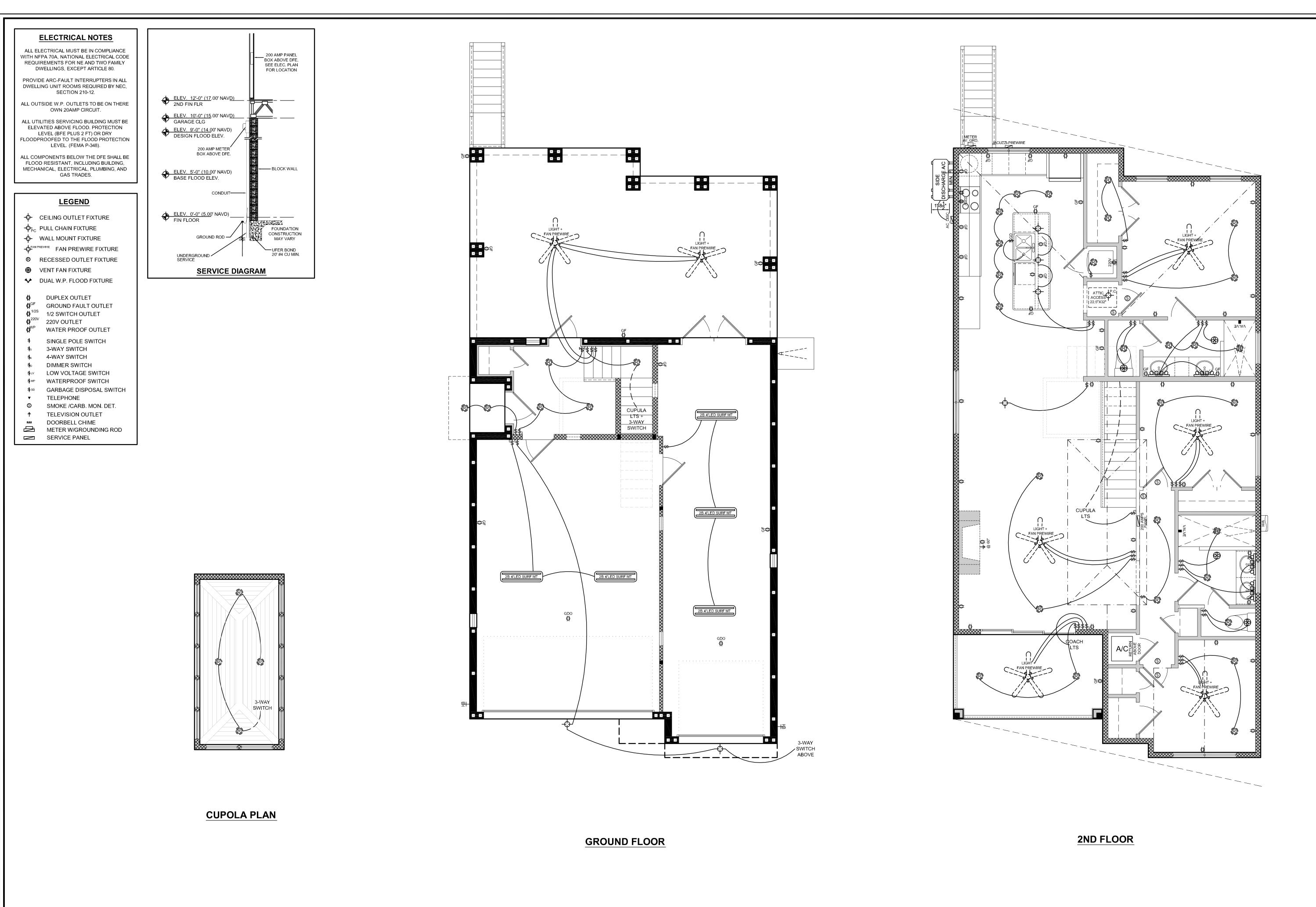
WOOD-FRAME CONSTRUCTION.

6. AT WALL AND ROOF INTERSECTIONS.

7. AT BUILT-IN GUTTERS.

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ELECTRICAL

SCALE: 1/4" = 1'

SCALE: 1/4" = 1'

SCALE: 1/4" = 1'

THIS SHEE ENGINE ENGINE

MASONRY OPENINGS

CONTRACTOR TO VERIFY REQUIRED MASONRY OPENINGS WITH DOOR/WINDOW SUPPLIER AND PROVIDE SLAB/MASON SUBS WITH M.O.'S PRIOR TO STARTING FOUNDATION

SETBACK VERIFICATION

A FOUNDATION SURVEY SHALL BE PERFORMED AND A COPY OF THE SURVEY SHALL BE ON SITE FOR THE BUILDING INSPECTORS USE, OR ALL PROPERTY MARKERS SHALL BE EXPOSED AND STRING STRETCHED FROM MARKER TO MARKER TO VERIFY REQUIRED SET BACKS

CONCRETE SLAB

SLAB TO BE 4" MIN. DEPTH, 3000 PSI CONCRETE WITH FIBER MESH OVER 6 MIL POLY FILM VAPOR BARRIER (LAPPED 12" & TAPED) ON CLEAN, COMPACTED 2000PSF MIN. (95% MODIFIED PROCTOR) SOIL WITH APPROVED TERMITE TREATMENT.

GENERAL NOTES

ALL DOOR RECESSES SHALL HAVE 1/8" PER FOOT SLOPE TOWARD EXTERIOR

FBCR 404.1.6: FINISHED SLAB FLOOR TO BE 4" MIN. ABOVE SURROUNDING GRADE. CHECK SURVEY FOR OTHER FINISHED FLOOR REQUIREMENTS.

ALL DIMENSIONS TO BE FIELD VERIFIED.

DIMENSIONS FOR WINDOWS ARE "GENERIC" AND USED FOR DESIGN PURPOSES ONLY,

VERIFY ALL WINDOW OPENINGS WITH WINDOW MANUFACTURER FOR EXACT ROUGH

OPENING SIZES.

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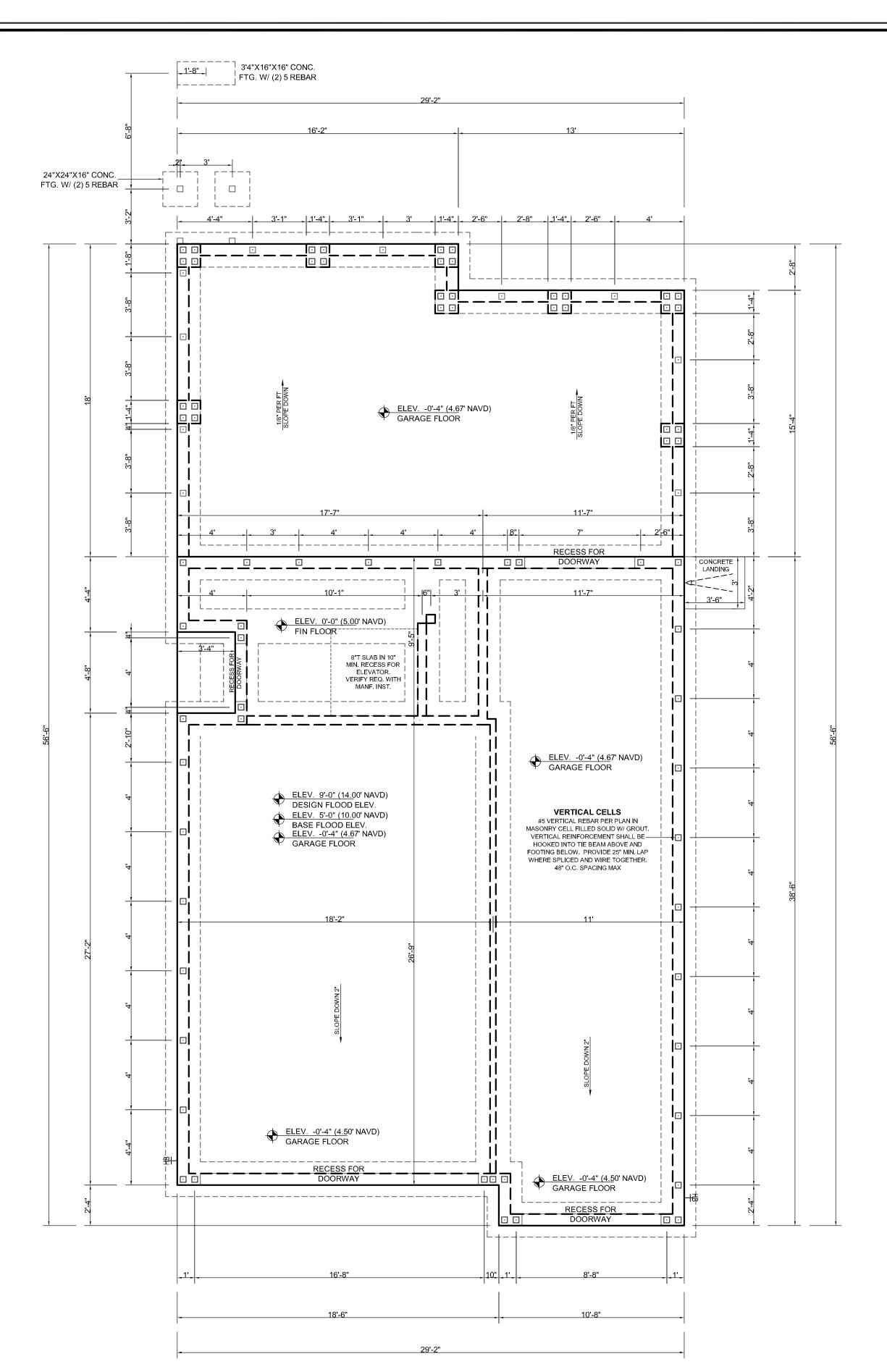
PLEASE REVIEW PLANS CAREFULLY PRIOR TO CONSTRUCTION AND COORDINATE WITH FINAL TRUSS DRAWINGS TO DETERMINE FINAL STRUCTURAL LAYOUT.

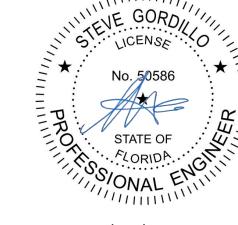
DO NOT USE STRUCTURAL DRAWINGS FOR BUILDING LAYOUT. COORDINATE LOCATIONS OF ALL STRUCTURAL ELEMENTS, INCLUDING COLUMNS, BEAMS, WALLS, SLABS, FOOTERS, CONNECTORS, AND BEARING REQUIREMENTS WITH ARCHITECTURAL DRAWINGS.

IT IS THE BUILDERS RESPONSIBILITY TO RESOLVE ANY CONFLICTS BETWEEN STRUCTURAL CONDITIONS AND ARCHITECTURAL DRAWINGS PRIOR TO LAYOUT AND CONSTRUCTION AND NOTIFY BOTH ENGINEER AND ARCHITECT IN WRITING PRIOR TO CONSTRUCTION.

ENGINEER IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OF THE BUILDER, IF ANY DETAIL OR SPECIFICATION IS NOT COMPLETELY CLEAR TO THE BUILDER PRIOR TO CONSTRUCTION, NOTIFY THE ENGINEER IN WRITING PRIOR TO CONSTRUCTION.

ADDITIONAL PLAN REVISIONS MAY NOT HAVE BEEN COMMUNICATED TO ENGINEER SINCE SIGN/SEAL DATE, IT IS THE BUILDERS RESPONSIBILITY TO RESOLVE ANY CONFLICTS BETWEEN STRUCTURAL CONDITIONS AND ARCHITECTURAL DRAWINGS PRIOR TO LAYOUT AND CONSTRUCTION AND NOTIFY BOTH ENGINEER AND ARCHITECT IN WRITING.





11/09/2024

A permit issued shall be construed to be a indense to proce work and not as authority to violate, cancel, alter or set asi provisions of the technical codes, nor shall issuance of a per the building official from thereafter requiring a correction of plans, construction or violations of this code. Every permit become invalid unless the work authorized by such per commenced within six months after its issuance, or if it outhorized by such permit is suspended or abandoned for a months after the time the work is commenced.

ANY UNAUTHORIZED USE, REPRODUCTION OR DUPLI THESE DRAWINGS WITHOUT THE THE EXPRESS W CONSENT OF THE BUILDER, DESIGNER AND ENGIN STRICTLY PROHIBITED

NOT SCALE DIMENSIONS FOR CONSTRUCTION PUI THE EVENT THAT A DIMENSION IS UNCLEAR OR M CONTACT THE ENGINEER IN WRITING

TURAL PORTIONS
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VOT CONSIDERED
NATURE MUST BE
SOPIES

FOR BASIC WIND SPEED OF 150 MPH, EXPOSURE"
THE DRAWING IS SEALED FOR THE STRUCTURAL POR ONLY. ALL OTHER ELEMENTS, SYSTEMS AND ASSEMBLI THE RESPONSIBILITY OF THE BUILDER
THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEES STEVE GORDILLO, PE USING A DIGITAL SIGNATURE AND SIGNED AND SEALED AND THE DIGITAL SIGNATURE MISSIGNED AND SEALED AND THE DIGITAL SIGNATURE MISSIGNED AND SEALED ON ANY ELECTRONIC COPIES

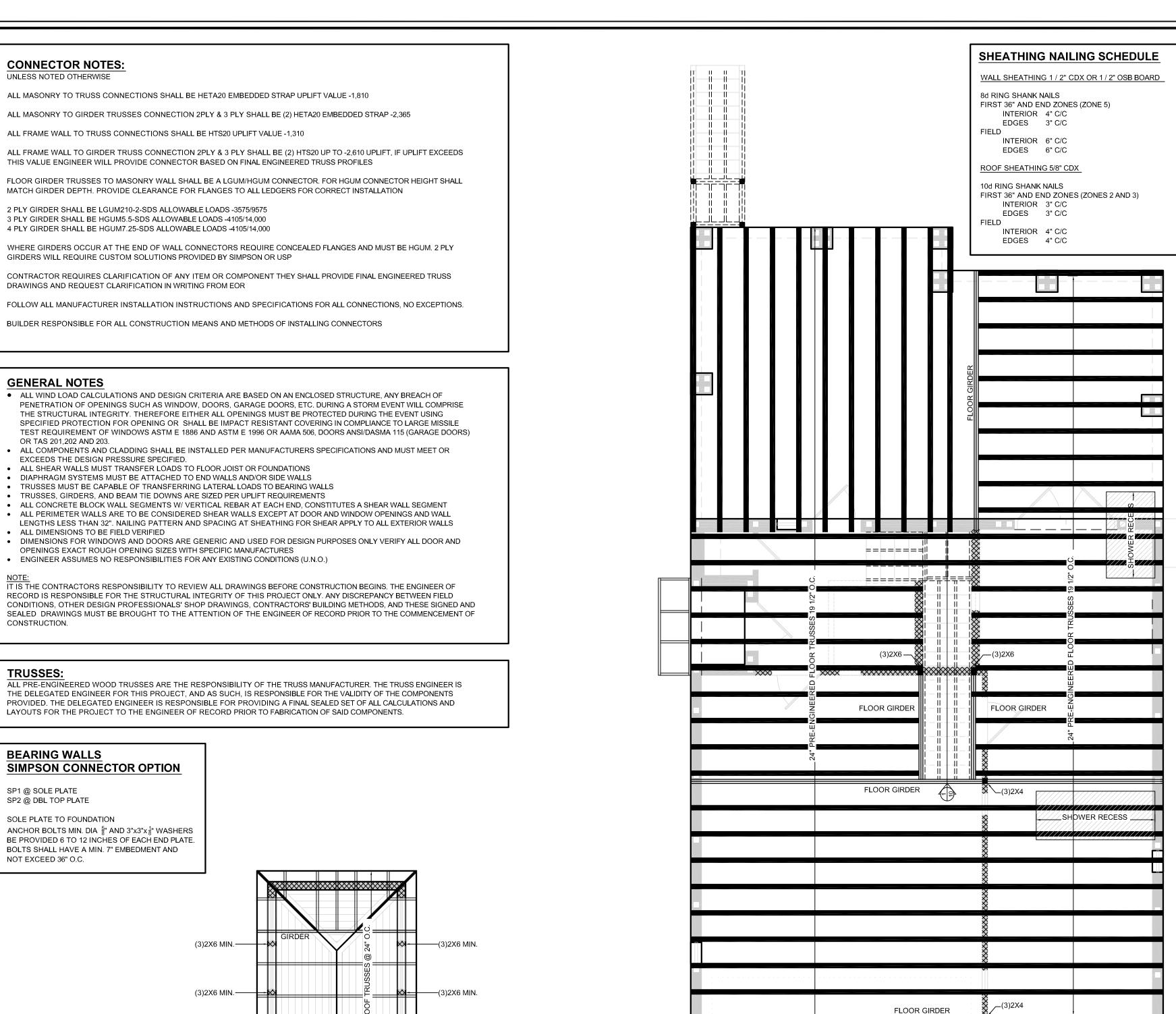
G3X DESIGN, LLC 2237 CLIMBING IVY DR TAMPA, FL 33618 (813) 928-8339 FL C.A. #31107

G3X1 2237 CL) TAM TAM (81

MORGANCASTLE

DATE REVISION DESIGNER
1-8-24 FINALS FOR PERMIT GM

SHEET CALISTRI
OF 7



11/09/2024 GIRDER SIMPSON BA212-2 (2)2X12 PT WOOD BEAL

CUPOLA PLAN

HEADER OVER OPENINGS

R302.12 DRAFTSTOPPING.

IN COMBUSTIBLE CONSTRUCTION WHERE THERE IS USABLE SPACE BOTH ABOVE AND BELOW THE CONCEALED SPACE OF A FLOOR/CEILING ASSEMBLY, DRAFTSTOPS SHALL BE INSTALLED SO THAT THE AREA OF THE CONCEALED SPACE DOES NOT EXCEED 1,000 SQUARE FEET (92.9 M2).

(3)2X6 MIN.-

(3)2X6 MIN.-

(3)2X8 STRUCTURAL HEADER OVER OPENINGS

DRAFTSTOPPING SHALL DIVIDE THE CONCEALED SPACE INTO APPROXIMATELY EQUAL AREAS. WHERE THE ASSEMBLY IS ENCLOSED BY A FLOOR MEMBRANE ABOVE AND A CEILING MEMBRANE BELOW, DRAFT STOPPING SHALL BE PROVIDED IN FLOOR/CEILING ASSEMBLIES UNDER THE FOLLOWING CIRCUMSTANCES:

1. CEILING IS SUSPENDED UNDER THE FLOOR FRAMING. 2. FLOOR FRAMING IS CONSTRUCTED OF TRUSS-TYPE OPEN-WEB OR PERFORATED MEMBERS.

302.12.1 MATERIALS.

ANY REPRODUCTION OR UNAUTHORIZED USE IS PROHIBITED.

DRAFTSTOPPING MATERIALS SHALL NOT BE LESS THAN 1/2-INCH (12.7 MM) GYPSUM BOARD, 3/8-INCH (9.5 MM) WOOD STRUCTURAL PANELS OR OTHER APPROVED MATERIALS ADEQUATELY SUPPORTED. DRAFTSTOPPING SHALL BE INSTALLED PARALLEL TO THE FLOOR FRAMING MEMBERS UNLESS OTHERWISE APPROVED BY THE BUILDING OFFICIAL. THE INTEGRITY OF THE DRAFTSTOPS SHALL BE MAINTAINED.

G3XDESIGN, LLC RESERVES THE RIGHT TO MAKE SUBSTITUTIONS TO ANY CONNECTOR SPECIFIED AFTER SUBMITTAL OF FINAL SIGNED AND SEALED TRUSS DRAWINGS HAVE BEEN PROVIDED FOR REVIEW.

BUILDER SHALL COORDINATE WITH G3XDESIGN, LLC PRIOR TO CONSTRUCTION

FINAL APPROVED TRUSS DRAWINGS MAY REQUIRE ADDITIONAL FOUNDATION SUPPORTS, COLUMNS, AND BEAMS NOT SHOWN ON PERMIT PLANS. BUILDER IS RESPONSIBLE FOR ANY REVISIONS PRIOR TO FINAL TRUSS APPROVAL

CONTRACTOR TO COORDINATE ALL VERTICAL AND HORIZONTAL FRAMING MEMBERS PRIOR TO CONSTRUCTION.

G3XDESIGN MAY REVISE HANGERS AFTER TRUSS DRAWING SHOP REVIEW.

FILLED CELLS WITH A MINIMUM 1-#5 BAR SHALL BE PROVIDED AT ALL LOCATIONS WHERE GIRDER AND GIRDER TRUSSES BEAR ON MASONRY WALLS

TYPICAL FRAME HEADER (LOAD BEARING WALL) ROUGH OPENING 2x4 FRAME WALL 2x6 FRAME WALL MIN (2) 2x12 W/ ½" PLYWOOD FLITCH MIN (3) 2x12 W/ 1/2" PLYWOOD FLITCH MIN (2) 2x12 W/ ½" PLYWOOD FLITCH MIN (3) 2x12 W/ 1/2" PLYWOOD FLITCH 6'-0" TO 8'-0" MIN (2) 2x12 W/ ½" PLYWOOD FLITCH MIN (3) $2x12 \text{ W} / \frac{1}{2}$ " PLYWOOD FLITCH OVER 8'-0" MIN 2 PCS OF 1 3/4" x 11 1/4" LVL BEAM MIN 3 PCS OF 1 $\frac{3}{4}$ " x 11 $\frac{1}{4}$ " LVL BEAM

2ND FLOOR

TRUSS LAYOUT

GROUND FLOOR

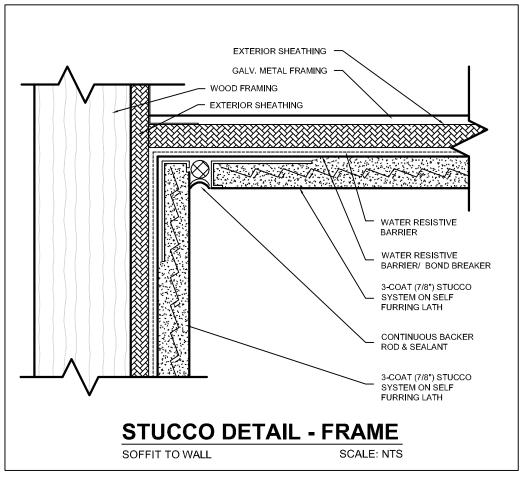
FRAMING NOTES: 1. ALL WOOD FRAMING EXPOSED TO THE EXTERIOR OR IN CONTACT WITH MASONRY OR CONCRETE IS TO BE PRESSURE 2. ALL EXTERIOR FASTENERS INCLUDING NAILS, HANGERS, BOLTS ETC. ARE TO BE STAINLESS STEEL (SS) TYPE 316 OR CORROSION RESISTANT 3. ALL INTERIOR FASTENERS IN CONTACT WITH PRESSURE TREATED LUMBER INCLUDING NAILS, HANGERS, BOLTS ETC.ARE TO BE HOT DIPPED GALVANIZED (HDG) G185.

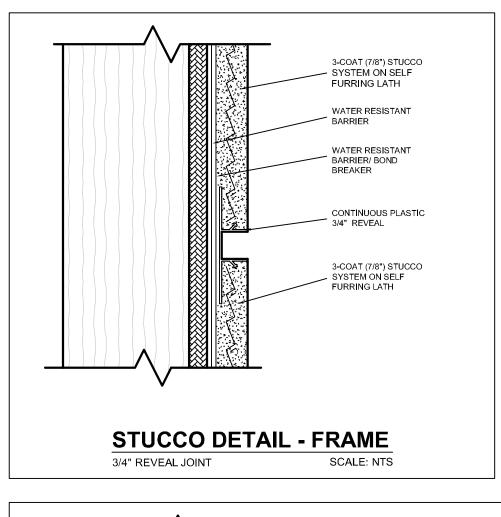
STUCCO BEAD NAILING PATTERN

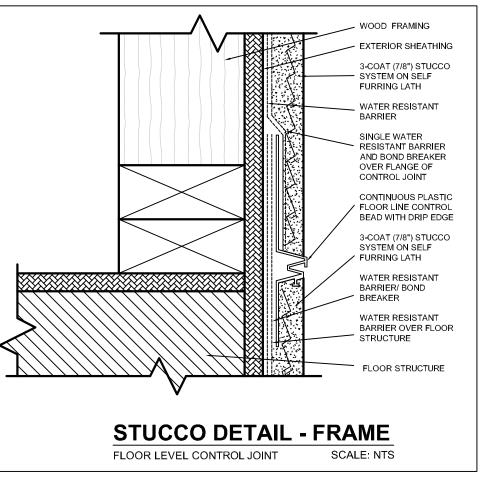
THE NAIL PATTERN FOR ALL STUCCO CORNER BEADS, $\frac{3}{4}$ " REVEAL JOINTS, FLOOR LEVEL CONTROL JOINTS, CONTROL JOINTS, INSIDE CORNERS AND OUTSIDE CORNERS SHALL BE NO MORE THAN 10" ON CENTER VERTICALLY AND HORIZONTALLY.

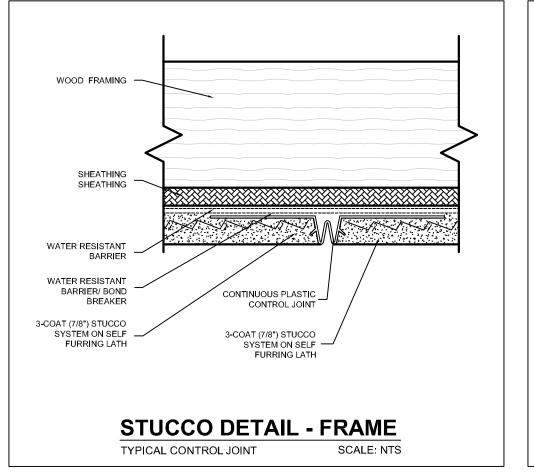
METAL LATH FASTENERS

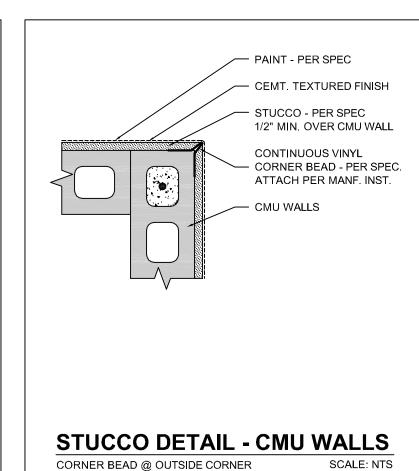
- 1. INSTALLATION OF LATH TO BE ACCORDING TO ASTM C1063.
- 2. LATH SHALL BE FASTENED TO WOOD STRUCTURAL PANELS WITH CORROSION RESISTANT 1- $\frac{1}{2}$ " LONG X $\frac{3}{4}$ " CROWN, 16 GAGE STAPLES DIRECT TO THE SHEATHING PANEL SPACED NOT MORE THAN 6-INCHES ON CENTER VERTICALLY AND HORIZONTALLY.
- 3. FASTENERS HAVE A WITHDRAWAL CAPACITY OF=90 PSF AND ARE ACCEPTABLE FOR WIND SPEEDS UP TO 170MPH, EXP "C".
- 4. THE STAPLE PATTERN FOR ALL STUCCO CORNER BEADS, $\frac{3}{4}$ REVEAL JOINTS, FLOOR LEVEL CONTROL JOINTS, CONTROL JOINTS, INSIDE CORNERS AND OUTSIDE CORNERS SHALL BE NO MORE THAN 6" ON CENTER VERTICALLY AND HORIZONTALLY.



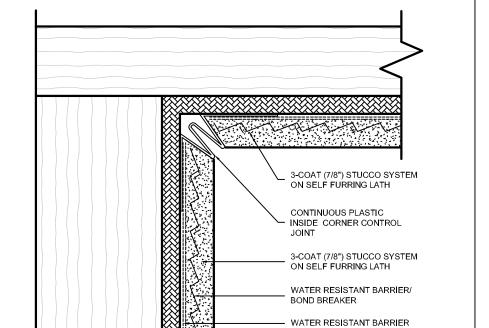








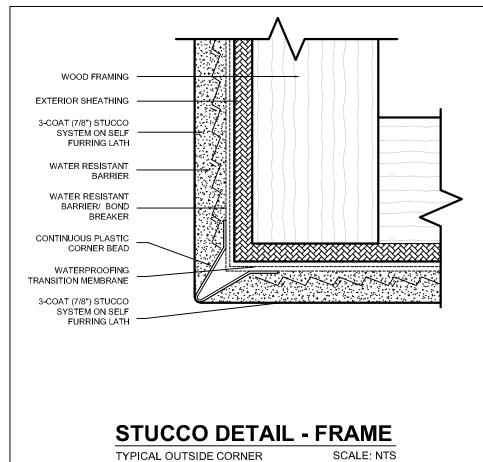
STUCCO NOTES

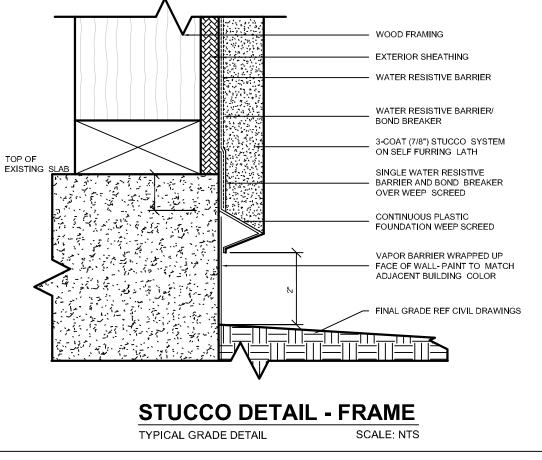


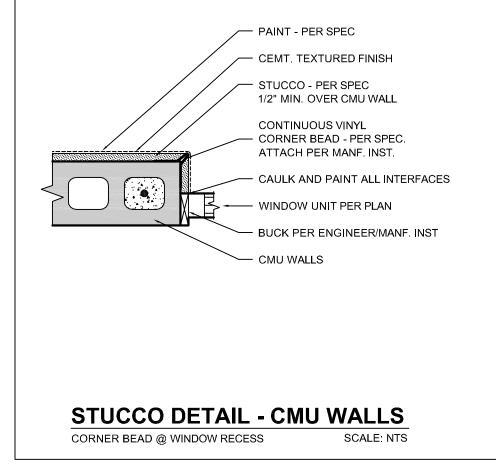
STUCCO DETAIL - FRAME TYPICAL INSIDE CORNER SCALE: NTS

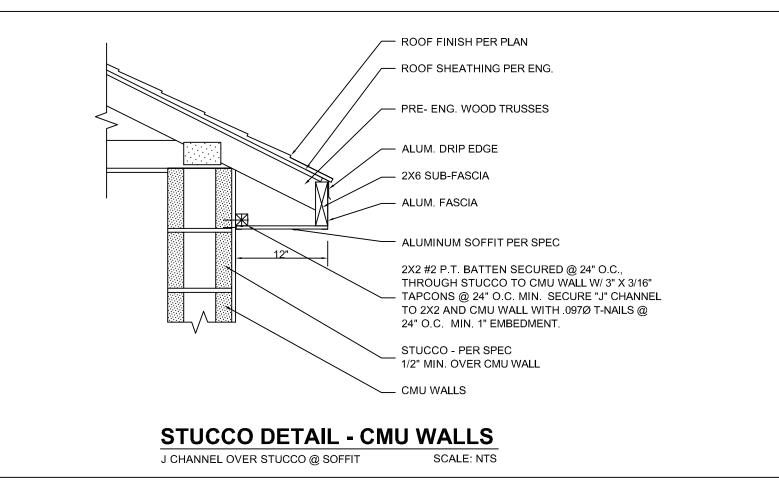
WOOD FRAMING

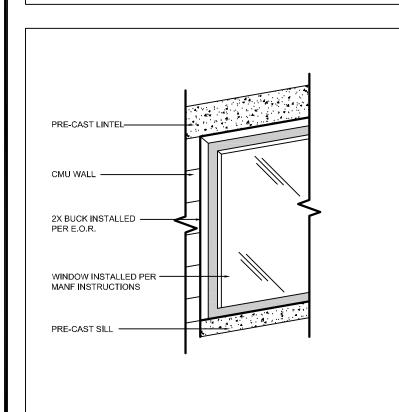
- EXTERIOR SHEATHING



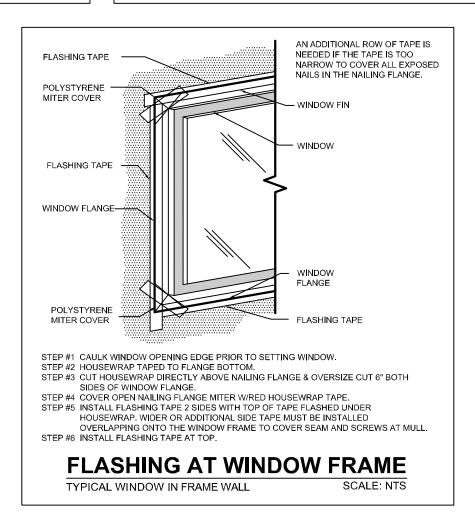


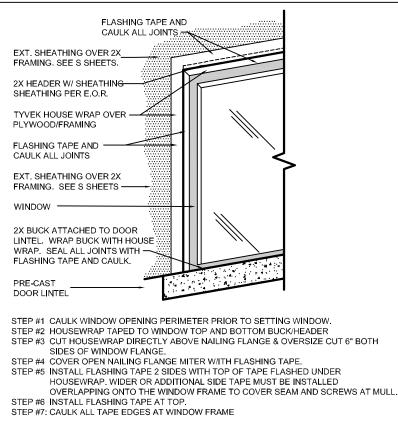


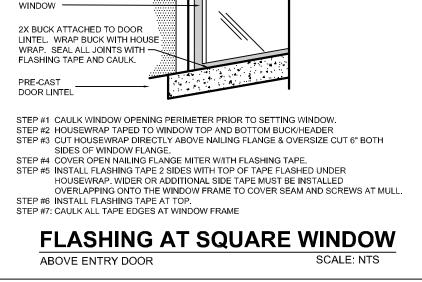


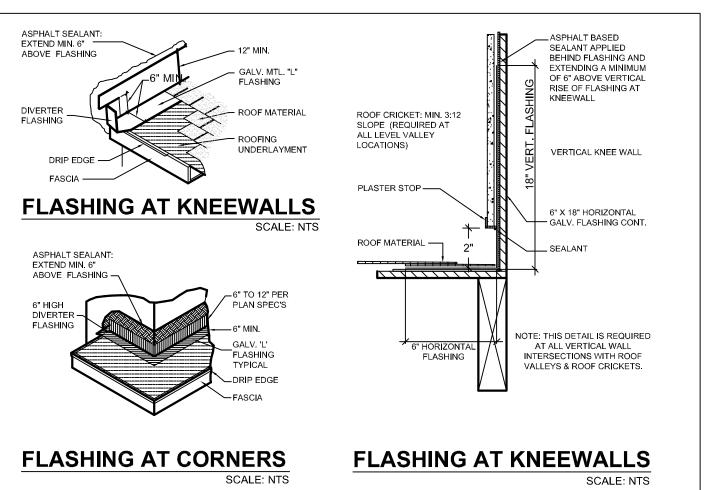


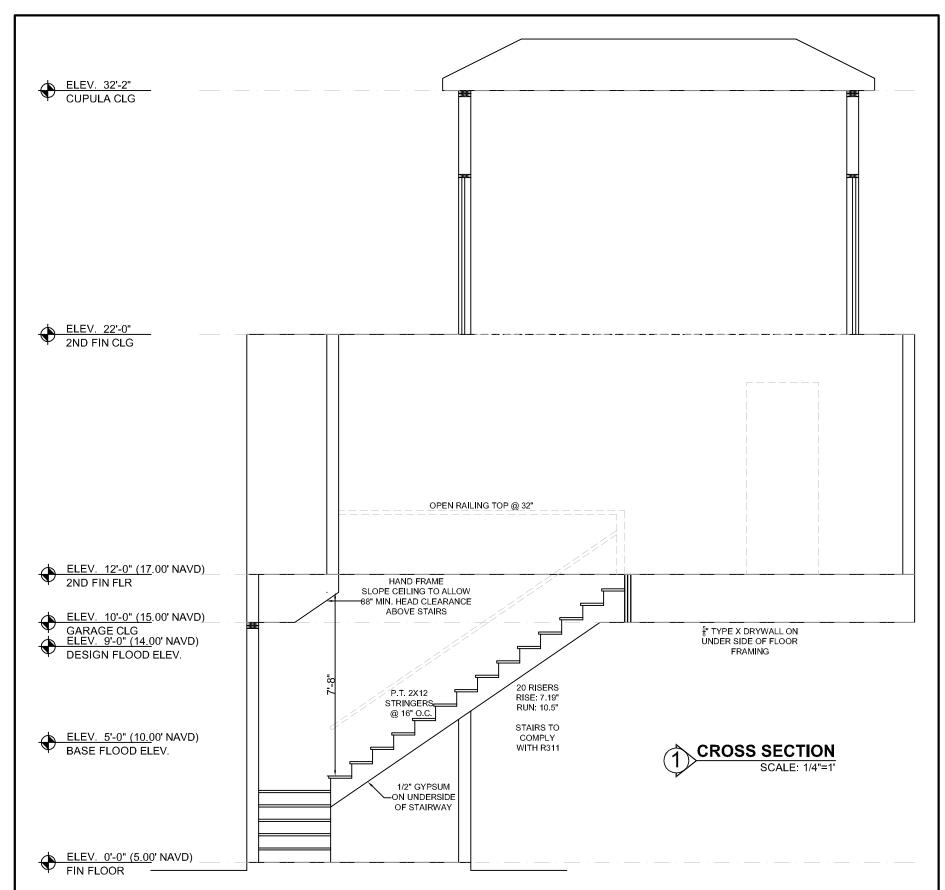
CAULKING AT WINDOW FRAME

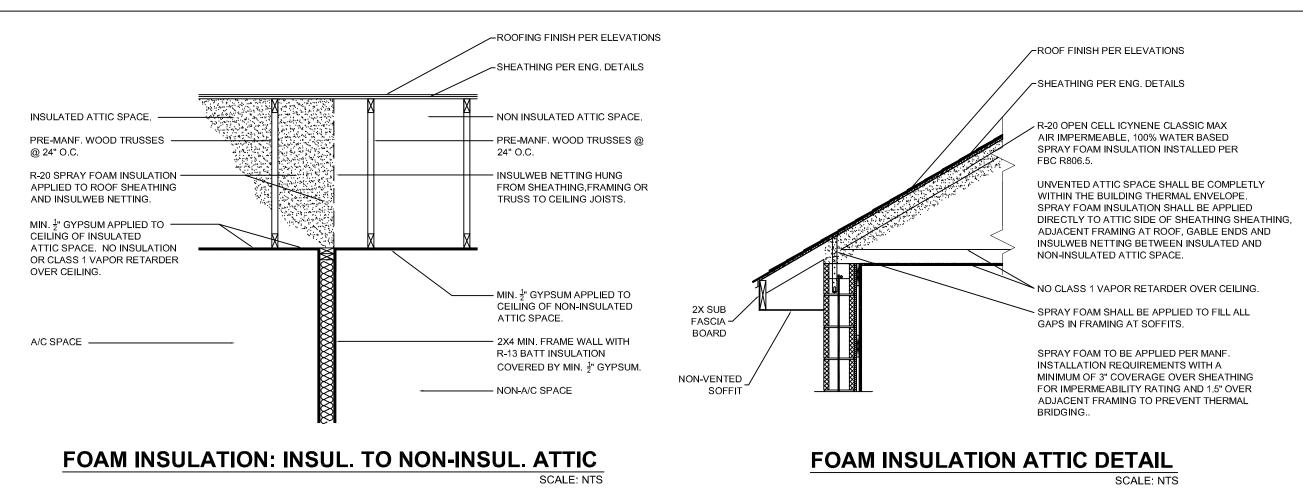


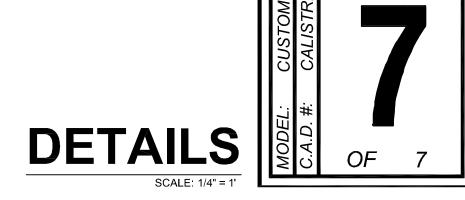






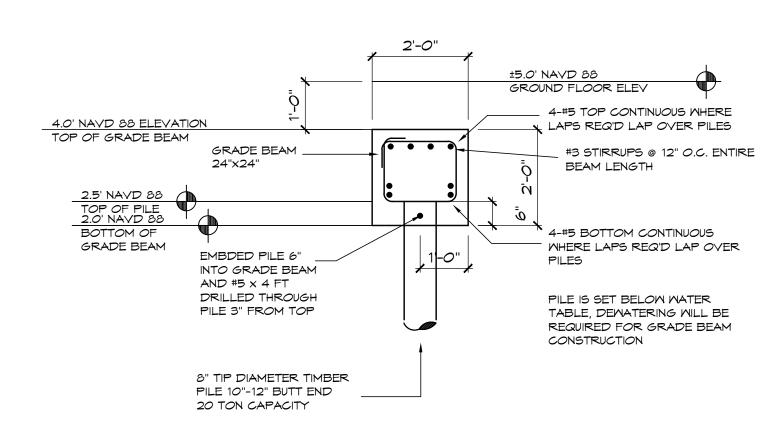






ENG

SHEET



MOOD PILES ARE TO BE 8" TIP DIAMETER TREATED PILES IN ACCORDANCE WITH ASTM D25 STANDARD SPECIFICATIONS FOR ROUND TIMBER PILES. ALLOWWABLE UNIT STRESSES FOR PILES ARE TO BE AS FOLLOMS:

FC = COMPRESSION PARRALLEL TO GRAIN = 1200 PSI Fb = EXTREME FIBER IN BENDING = 2400 PSI Fy = HORIZONTAL SHEAR = 110 PSI

FCP= COMPRESSION PERPENDICULAR TO GRAIN = 250 PSI E = MODULUS OF ELASTICITY = 1,500,000 PSI

PILES ARE TO BE DRIVEN TO PRACTICAL REFUSAL OR UNTIL PILE CAN DEVELOP A MINIMUM CAPACITY OF 20 TONS

PILES ARE TO BE CAPABLE OF RESISTING 2 TON LATERAL LOAD

PILES TO BE LOAD TESTED IF NOT DRIVEN TO REFUSAL

PILE SPLICES ARE TO BE APPROVED BY A LICENSED GEOTECHNICAL ENGINEER PRIOR TO DRIVING PILES

PILE AND GRADE BEAM PLAN

SCALE: 1/4 "=1'-@"

PLEASE REFER TO ORIGINAL REPORT OF SUBSURFACE EXPLORATION FOR SOIL DATA AND PILE RECOMMENDATIONS

NOTES PROVIDED HEREIN ARE PROVIDED FOR REFERENCE RELATED TO THE FOUNDATION/PILE/GRADE BEAM DESIGNED FOR THIS PROJECT. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR GEOTECHINCAL RECOMMENDATIONS OR PRIOR TEST RESULTS.

ENGINEER OF RECORD RECOMMENDS RETAINING ORIGINAL GEOTECHINCAL ENGINEER DURING CONSTRUCTION FOR ALL GEOTECHINCAL RELATED ACTIVITIES.

ALL DIMENSIONS TO BE FIELD VERIFIED.

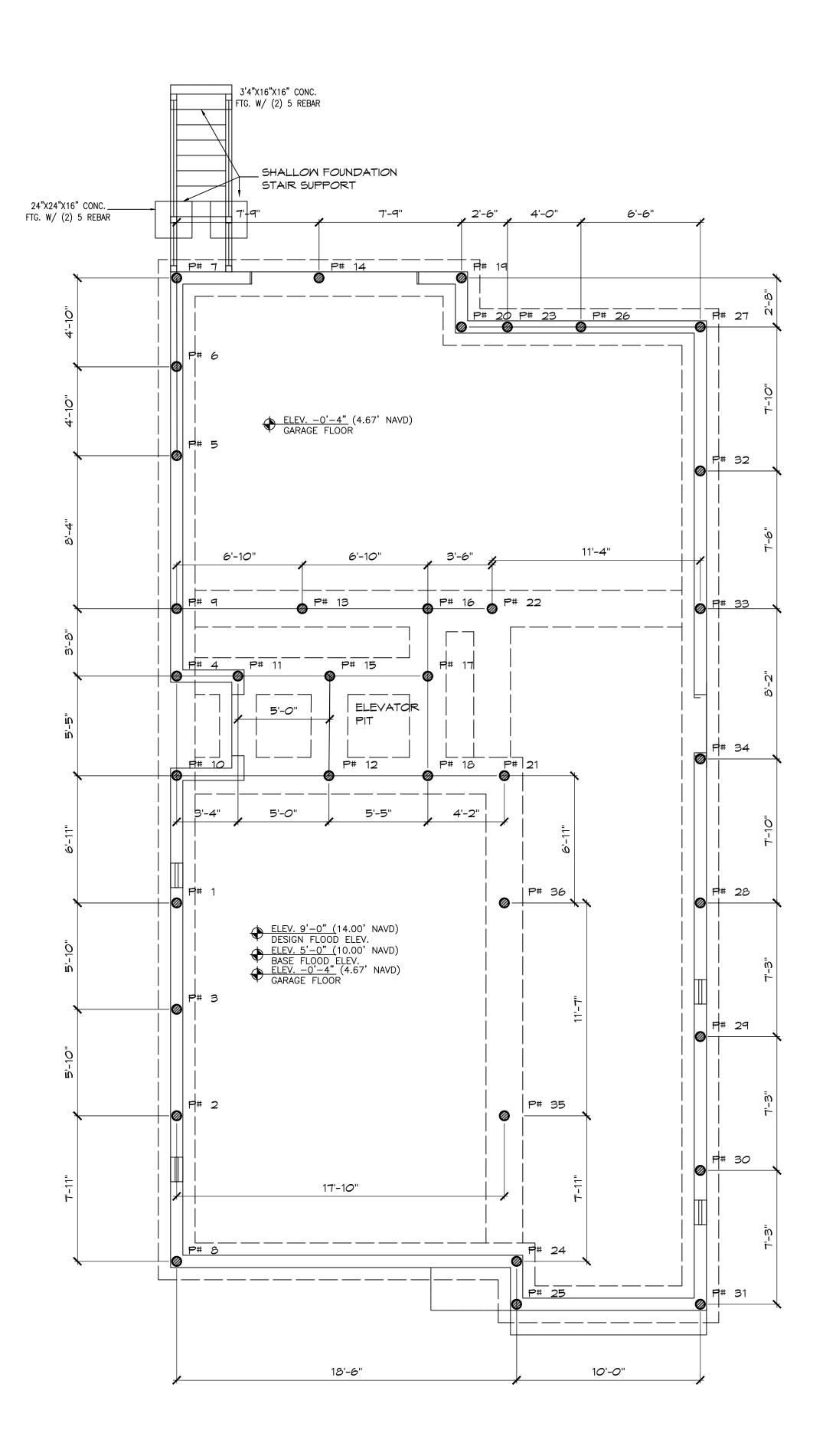
PILE LOCATIONS ARE APPROXIMATE AND CAN BE

ADJUSTED IN THE FIELD ±2"

PILES SHALL BE CENTERED ON GRADE BEAM

8" CMU STEMWALL ABOVE GRADE BEAM SHOWN AS REFERENCE

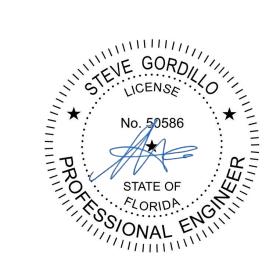
TOP OF GRADE BEAM SHALL BE A MINIMUM 12" BELOW LOWEST EXISTING GRADE



A permit issued shall be construed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

ANY UNAUTHORIZED USE, REPRODUCTION OR DUPLICATION OF THESE DRAWINGS WITHOUT THE THE EXPRESS WRITTEN CONSENT OF THE BUILDER, DESIGNER AND ENGINEER IS STRICTLY PROHIBITED

DO NOT SCALE DIMENSIONS FOR CONSTRUCTION PURPOSES. IN THE EVENT THAT A DIMENSION IS UNCLEAR OR MISSING CONTACT THE ENGINEER IN



NOVEMBER 8, 2024

I CERTIFY THAT TO THE BEST OF THE ENGINEER'S KNOWLEDGE AND BELIEF ALL OF THE STRUCTURAL ELEMENTS AND SYSTEMS HAVE BEEN DESIGNED TO BE IN COMPLIANCE WITH THE 8TH EDITION OF THE 2023 RESIDENTIAL FLORIDA BUILDING CODE FOR BASIC WIND SPEED OF 150 MPH, EXPOSURE "D".

THE DRAWING IS SEALED FOR THE STRUCTURAL PORTIONS ONLY. ALL OTHER ELEMENTS, SYSTEMS AND ASSEMBLIES ARE THE RESPONSIBILITY OF THE

THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY STEVE GORDILLO, PE USING A DIGITAL SIGNATURE AND DATE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE DIGITAL SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES

ENGINEERING <u>DESIGN</u>

G3X DESIGN, LLC 2237 CLIMBING IVY DR TAMPA, FL 33618 (813) 928-8339 FL C.A. #31107



RESIDENCE 13222 3RD STREET EAST MADERIA BEACH, FL

OMAR ABBAS

ABBAS DEVELOPMENT **BUILDING CONTRACTOR** 210 S PINELLAS AVE SUITE 220 727-946-0475

DESIGNER

Curtis Morgan

Morgancastle Studio, Inc. Residential Design Services

9324 Wildwood Ave. Hudson, FL 34669 Phone: (727)247-8148

morgancastlestudio@gmail.com

REVISIONS

DATE

11-08-2024

FINAL PERMIT

PRE-ENGINEERED WOOD ROOF TRUSSES @ 24" O.C. SEE ENGINEERING W/ SIMPSON H10 CONNECTOR. DIMENSIONAL F.G. SHINGLES (SEE MANUFACTURER'S INSTALLATION INSTRUCTION & TEST DATA) OVER 5/8" CDX PLYWOOD SHEATHING .-PEEL AND STICK REQUIREMENT (R905.2.7) ENTIRE ROOF DECK SHALL BE COVERED WITH AN APPROVED SELF-ADHERING POLYMER MODIFIED BITUMEN SHEET MEETING ASTM D 1970 OR AN APPROVED SPRAY FOAM INSULATION SELF-ADHERING SYNTHETIC UNDERLAYMENT INSTALLED IN M/ MINIMUM R-20 VALUE. ACCORDANCE WITH THE MANUFACTURER'S INSTALLATION SEE ARCHITECTURAL DETAILS FOR SPECIFIC OVERHANG AND SOFFIT STRUCTURAL 2x6 STUD WALLS @ 16" O.C. DETAILS, CONDITION DEPICTED IS A HEADER 1/2" ZIP BOARD PER SHEATHING GENERAL STRUCTURAL CONDITION NAILING SCHEDULE DOUBLE TOP PLATE SP2 SINGLE BOTTOM PLATE W SP1 DOUBLE STUDS ALL OPENINGS PLATE TO TRUSS MIN R-19 INSULATION USING SDMC15600 @ TRUSS SPACING DIMENSIONAL F.G. SHINGLES (SEE MANUFACTURER'S INSTALLATION INSTRUCTION & TEST DATA) OVER 5/8" CDX PLYWOOD SHEATHING. -PRE-ENGINEERED WOOD ROOF TRUSSES @ 24" O.C. SEE ENGINEERING W/ SIMPSON HTS20 CONNECTOR. SPRAY FOAM INSULATION M/ MINIMUM R-20 VALUE. MAINTAIN CONTINUITY INSULATION AT SOFFIT PER CODE REQUIREMENT. SEE ARCHITECTURAL DETAILS FOR SPECIFIC OVERHANG AND SOFFIT T - 1/2" DRYWALL DETAILS, CONDITION DEPICTED IS A GENERAL STRUCTURAL CONDITION 2X BLOCKING @ 2x6 STUD WALLS @ 16" O.C. 5" ZIP BOARD PER SHEATHING NAILING SCHEDULE DOUBLE TOP PLATE SP2 SINGLE BOTTOM PLATE W SP1, DOUBLE STUDS ALL OPENINGS MIN R-19 INSULATION 2x BLOCKING @ 4'-0" DECORATIVE CEMENTITIOUS FINISH 3 COATS & FINISHED THICKNESS STUCCO SHALL BE IN COMPLIANCE WITH ASTM C926. ASTM C1063 AND PROVISIONS R703.7.1 THRU R703.7.5 SEE ELEVATIONS FOR EXACT FINISH SP1 @ SOLE 24" PRE-ENGINEERED FLOOR PLATE TO TRUSS TRUSS SYSTEM W/ 3/4" USING SDMC15600 @ TRUSS SPACING ADVANTECH PLYWOOD DECK MSTAM36 STRAP @ 32" *O.*C. HETA20 EA TRUSS TO CMU -D.F.E. 14.00' N.A.V.D. 5/8" TYPE 'X' DRYWALL PER FBC 2023 R302.5.\$ R302.6 BETWEEN GARAGE 16" HIGH FILLED PERIMETER & LIVING AREA.) BOND BEAM, 16" HIGH W/ 1#50 CONTINUOUS -25" LAP CORNER BARS ALL BUILDING MATERIALS BELOW THE DESIGN FLOOD ELEVATION MUST BE FLOOD DAMAGE-RESISTANT PER FEMA TB2-AUGUST 2008 ONLY CLASS 4 AND CLASS 5 MATERIALS ARE ACCEPTABLE FOR AREAS BELOW THE DESIGN FLOOD B.F.E. 10.00' N.A.V.D. AE-10 ELEVATION (DFE). MATERIALS THAT ARE NOT FOOD DAMAGE-RESISTANT MATERIALS, SUCH AS WIRING FOR FIRE ALARMS AND EMERGENCY LIGHTING, ARE ALLOWED BELOW THE DFE IF SPECIFICALLY REQUIRED TO ADDRESS LIFE SAFETY AND ELECTRIC CODE REQUIREMENTS FOR BUILDING ACCESS AND DECORATIVE CEMENTITIOUS FINISH STORAGE AREAS. 3 COATS & FINISHED THICKNESS STUCCO SHALL BE IN COMPLIANCE WITH ASTM C926, ASTM C1063 AND PROVISIONS 4" 3000 P.S.I. CONCRETE R703.7.1 THRU R703.7.5 OR SLAB W/ FIBERMESH OVER 6 MIL. VISQUEEN AND CLEAN COMPACTED 4"x4" INSPECTION PORT AT TERMITE TREATED FILL. FILLED CELL F.F.E = 4.67'

DO NOT USE STRUCTURAL DRAWINGS FOR BUILDING LAYOUT. COORDINATE LOCATIONS OF ALL STRUCTURAL ELEMENTS, INCLUDING COLUMNS, BEAMS, WALLS, SLABS, FOOTERS, CONNECTORS, AND BEARING REQUIREMENTS WITH ARCHITECTURAL DRAWINGS.

IT IS THE BUILDERS RESPONSIBILITY TO RESOLVE ANY CONFLICTS BETWEEN STRUCTURAL CONDITIONS AND ARCHITECTURAL DRAWINGS PRIOR TO LAYOUT AND CONSTRUCTION AND NOTIFY BOTH ENGINEER AND ARCHITECT IN MRITING.

BEARING WALLS SIMPSON CONNECTOR OPTION

SP1 @ SOLE PLATE

SP2 @ DBL TOP PLATE

SOLE PLATE TO FOUNDATION ANCHOR BOLTS MIN. DIA & AND 3"x3"x3" WASHERS BE PROVIDED 6 TO 12 INCHES OF EACH END PLATE. BOLTS SHALL HAVE A MIN. 7" EMBEDMENT AND NOT EXCEED 36" O.C.

PLEASE REVIEW PLANS CAREFULLY PRIOR TO CONSTRUCTION AND COORDINATE WITH FINAL TRUSS DRAWINGS TO DETERMINE FINAL STRUCTURAL LAYOUT.

IT IS THE BUILDERS RESPONSIBILITY TO NOTIFY BOTH ENGINEER AND ARCHITECT OF ANY CONFLICTS OF ANY CONDITIONS.

DO NOT USE STRUCTURAL DRAWINGS FOR BUILDING LAYOUT. COORDINATE LOCATIONS OF ALL STRUCTURAL ELEMENTS, INCLUDING COLUMNS, BEAMS, WALLS, SLABS, FOOTERS, CONNECTORS, AND BEARING REQUIREMENTS WITH ARCHITECTURAL DRAWINGS.

IT IS THE BUILDERS RESPONSIBILITY TO RESOLVE ANY CONFLICTS BETWEEN STRUCTURAL CONDITIONS AND ARCHITECTURAL DRAWINGS PRIOR TO LAYOUT AND CONSTRUCTION AND NOTIFY BOTH ENGINEER AND ARCHITECT IN WRITING PRIOR TO CONSTRUCTION.

ENGINEER IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OF THE BUILDER, IF ANY DETAIL OR SPECIFICATION IS NOT COMPLETELY CLEAR TO THE BUILDER PRIOR TO CONSTRUCTION, NOTIFY THE ENGINEER IN WRITING PRIOR TO CONSTRUCTION

DUE TO CLARITY NOT ALL REQUIRED FLASHING IS INDICATED ON THE DRAWINGS. FLASHING SHALL BE INSTALLED PER FBC 2020 R703.4. CODE SECTION HAS BEEN PROVIDED BELOW AS REFERENCE ONLY

R703.4 FLASHING. APPROVED CORROSION-RESISTANT FLASHING SHALL BE APPLIED SHINGLE-FASHION IN A MANNER TO PREVENT ENTRY OF WATER INTO THE WALL CAVITY OR PENETRATION OF WATER TO THE BUILDING STRUCTURAL FRAMING COMPONENTS. SELF-ADHERED MEMBRANES USED AS FLASHING SHALL COMPLY WITH AAMA 711. ALL EXTERIOR FENESTRATION PRODUCTS SHALL BE SEALED AT THE JUNCTURE WITH THE BUILDING WALL WITH A SEALANT COMPLYING WITH AAMA 800 OR ASTM C920 CLASS 25 GRADE NS OR GREATER FOR PROPER JOINT EXPANSION AND CONTRACTION, ASTM C1281, AAMA 812, OR OTHER APPROVED STANDARD AS APPROPRIATE FOR THE TYPE OF SEALANT. FLUID-APPLIED MEMBRANES USED AS FLASHING IN EXTERIOR WALLS SHALL COMPLY WITH AAMA 714. THE FLASHING SHALL EXTEND TO THE SURFACE OF THE EXTERIOR WALL FINISH. APPROVED CORROSION-RESISTANT FLASHINGS SHALL BE INSTALLED AT THE FOLLOWING LOCATIONS:

1. EXTERIOR WINDOW AND DOOR OPENINGS. FLASHING AT EXTERIOR WINDOW AND DOOR OPENINGS SHALL EXTEND TO THE SURFACE OF THE EXTERIOR WALL FINISH OR TO THE WATER-RESISTIVE BARRIER COMPLYING WITH SECTION 703.2 FOR SUBSEQUENT DRAINAGE. MECHANICALLY ATTACHED FLEXIBLE FLASHINGS SHALL COMPLY WITH AAMA 712. FLASHING AT EXTERIOR WINDOW AND DOOR OPENINGS SHALL BE INSTALLED IN ACCORDANCE WITH ONE OR MORE OF THE FOLLOWING: 1.1. THE FENESTRATION MANUFACTURER'S INSTALLATION AND FLASHING

INSTRUCTIONS, OR FOR APPLICATIONS NOT ADDRESSED IN THE FENESTRATION MANUFACTURER'S INSTRUCTIONS. IN ACCORDANCE WITH THE FLASHING MANUFACTURER'S INSTRUCTIONS. WHERE FLASHING INSTRUCTIONS OR DETAILS ARE NOT PROVIDED, PAN FLASHING SHALL BE INSTALLED AT THE SILL OF EXTERIOR WINDOW AND DOOR OPENINGS. PAN FLASHING SHALL BE SEALED OR SLOPED IN SUCH A MANNER AS TO DIRECT WATER TO THE SURFACE OF THE EXTERIOR WALL FINISH OR TO THE WATER-RESISTIVE BARRIER FOR SUBSEQUENT DRAINAGE. OPENINGS USING PAN FLASHING SHALL INCORPORATE FLASHING OR PROTECTION AT THE HEAD AND SIDES.

1.2. IN ACCORDANCE WITH THE FLASHING DESIGN OR METHOD OF A REGISTERED DESIGN PROFESSIONAL. 1.3. IN ACCORDANCE WITH OTHER APPROVED METHODS.

1.4 IN ACCORDANCE WITH FMA/AAMA 100, FMA/AAMA 200, FMA/MDMA 250, FMA/AAMA/ WDMA 300 OR FMA/AAMA/WDMA 400.

2. AT THE INTERSECTION OF CHIMNEYS OR OTHER MASONRY CONSTRUCTION WITH FRAME OR STUCCO WALLS, WITH PROJECTING LIPS ON BOTH SIDES UNDER STUCCO UNDER AND AT THE ENDS OF MASONRY, MOOD OR METAL COPINGS AND SILLS.

CONTINUOUSLY ABOVE ALL PROJECTING WOOD TRIM. WHERE EXTERIOR PORCHES, DECKS OR STAIRS ATTACH TO A WALL OR FLOOR ASSEMBLY OF WOOD-FRAME CONSTRUCTION.

6. AT WALL AND ROOF INTERSECTIONS. 7. AT BUILT-IN GUTTERS.

ALL DIMENSIONS TO BE FIELD VERIFIED.

VERIFY ALL MINDOM OPENINGS MITH MINDOM MANUFACTURER FOR EXACT ROUGH

ALL GLAZED OPENINGS SHALL BE IMPACT

ALL PERIMETER WALLS ARE TO BE CONSIDERED SHEAR WALLS EXCEPT AT DOOR AND WINDOW OPENINGS AND WALL LENGTHS LESS THAN 2'-8". NAILING PATTERN AND SPACING AT SHEATHING FOR

FRAMING NOTES

1. ALL WOOD FRAMING EXPOSED TO THE EXTERIOR OR IN CONTACT WITH MASONRY OR CONCRETE IS TO BE PRESSURE TREATED (PT)

2. ALL EXTERIOR FASTENERS INCLUDING NAILS, HANGERS, BOLTS ETC. ARE TO BE STAINLESS STEEL (SS) TYPE 316 OR CORROSION RESISTANT

3. ALL INTERIOR FASTENERS IN CONTACT WITH PRESSURE TREATED LUMBER INCLUDING NAILS, HANGERS, BOLTS ETC.ARE TO BE HOT DIPPED GALVANIZED (HDG) G185.

SHEATHING NAILING SCHEDULE

MALL SHEATHING 1 / 2" CDX OR 1 / 2" OSB BOARD 8d RING SHANK NAILS

FIRST 36" AND END ZONES (ZONE 5) INTERIOR 4" C/C EDGES 3" C/C

> INTERIOR 6" C/C EDGES 6" C/C

ROOF SHEATHING 5/8" CDX

10d RING SHANK NAILS FIRST 36" AND END ZONES (ZONES 2 AND 3)

INTERIOR 3" C/C EDGES 3" C/C

INTERIOR 4" C/C

EDGES 4" C/C

ALL MASONRY TO TRUSS CONNECTIONS SHALL BE HETA20 EMBEDDED STRAP UPLIFT VALUE -1,810

ALL MASONRY TO GIRDER TRUSSES CONNECTION 2PLY \$ 3 PLY SHALL BE (2) HETA20 EMBEDDED STRAP -1,810

CONNECTOR NOTES:

UNLESS NOTED OTHERWISE

ALL FRAME WALL TO TRUSS CONNECTIONS SHALL BE HTS20 UPLIFT VALUE -1,310

ALL FRAME WALL TO GIRDER TRUSS CONNECTION 2PLY & 3 PLY SHALL BE (2) HTS20 UP TO -2,610 UPLIFT, IF UPLIFT EXCEEDS THIS VALUE ENGINEER WILL PROVIDE CONNECTOR BASED ON FINAL ENGINEERED TRUSS PROFILES

FLOOR GIRDER TRUSSES TO MASONRY WALL SHALL BE A LGUM/HGUM

CONNECTOR. FOR HGUM CONNECTOR HEIGHT SHALL MATCH GIRDER DEPTH. PROVIDE CLEARANCE FOR FLANGES TO ALL LEDGERS FOR CORRECT INSTALLATION

2 PLY GIRDER SHALL BE LGUM210-2-SDS ALLOWABLE LOADS -3575/9575 3 PLY GIRDER SHALL BE HGUM5.5-SDS ALLOWABLE LOADS -4105/14,000 4 PLY GIRDER SHALL BE HGUM7.25-SDS ALLOMABLE LOADS -4105/14,000

WHERE GIRDERS OCCUR AT THE END OF WALL CONNECTORS REQUIRE CONCEALED FLANGES AND MUST BE HGUM. 2 PLY GIRDERS WILL REQUIRE CUSTOM SOLUTIONS PROVIDED BY SIMPSON OR USP

CONTRACTOR REQUIRES CLARIFICATION OF ANY ITEM OR COMPONENT THEY SHALL PROVIDE FINAL ENGINEERED TRUSS DRAWINGS AND REQUEST CLARIFICATION IN WRITING FROM EOR

FOLLOW ALL MANUFACTURER INSTALLATION INSTRUCTIONS AND SPECIFICATIONS FOR ALL CONNECTIONS, NO EXCEPTIONS.

BUILDER RESPONSIBLE FOR ALL CONSTRUCTION MEANS AND METHODS OF INSTALLING CONNECTORS

UPLIFTS ARE NOT ANTICIPATED TO EXCEED 1,810 FOR TRUSSES AND 4,730 FOR GIRDER TRUSSES, UPON REVIEW OF FINAL TRUSS DRAWINGS PROVIDED BY THE TRUSS COMPANY, IF ANY VALUES EXCEED THESE STATED VALUES, PLEASE NOTIFY ENGINEER IN WRITING PRIOR TO CONSTRUCTION

UPLIFT STRAPS FOR COMMON TRUSSES SHALL BE HTS20 UPLIFT VALUE 1,420 & H10 FOR CUPLOA TRUSSES UPLIFT VALUE 1,040

UPLIFT STRAPS FOR GIRDER TRUSSES SHALL BE 2 PLY GIRDER TRUSS (2) HTS20 STRAPS = 2,840 OR MGT IF UPLIFT 3 PLY GIRDER TRUSS MGT = 4,365 OR HGT-3 IF UPLIFT EXCEEDS 4,365

IF UPLIFT VALUES EXCEEDS THESE VALUES PLEASE NOTIFY ENGINEER IN WRITING PRIOR TO CONSTRUCTION

DIMENSIONS FOR WINDOWS ARE "GENERIC" AND USED FOR DESIGN PURPOSES ONLY,

OPENING SIZES

RESISTANT.

SHEAR APPLY TO ALL EXTERIOR FRAME WALLS

I CERTIFY THAT TO THE BEST OF THE ENGINEER'S KNOWLEDGE AND BELIEF ALL OF THE STRUCTURAL ELEMENTS AND SYSTEMS HAVE BEEN DESIGNED TO BE IN COMPLIANCE WITH THE 8TH EDITION OF THE 2023 RESIDENTIAL FLORIDA BUILDING CODE FOR BASIC WIND SPEED OF 150 MPH, EXPOSURE "D".

NOVEMBER 8, 2024

A permit issued shall be construed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall

thereafter requiring a correction of errors in plans, construction

or violations of this code. Every permit issued shall become

commenced within six months after its issuance, or if the work

DUPLICATION OF THESE DRAWINGS WITHOUT THE THE

authorized by such permit is suspended or abandoned for a

period of six months after the time the work is commenced.

DESIGNER AND ENGINEER IS STRICTLY PROHIBITED

DO NOT SCALE DIMENSIONS FOR CONSTRUCTION

UNCLEAR OR MISSING CONTACT THE ENGINEER IN

PURPOSES. IN THE EVENT THAT A DIMENSION IS

LEVE GORDI

LICENSE

STATE OF

1,0'S/ONAL

FLORIDA

issuance of a permit prevent the building official from

invalid unless the work authorized by such permit is

ANY UNAUTHORIZED USE, REPRODUCTION OR

EXPRESS WRITTEN CONSENT OF THE BUILDER,

THE DRAWING IS SEALED FOR THE STRUCTURAL PORTIONS ONLY, ALL OTHER ELEMENTS, SYSTEMS AND ASSEMBLIES ARE THE RESPONSIBILITY OF THE

THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY STEVE GORDILLO, PE USING A DIGITAL SIGNATURE AND DATE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE DIGITAL SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES

ENGINEERING DESIGN

G3X DESIGN, LLC 2237 CLIMBING IVY DR TAMPA, FL 33618 (813) 928-8339 FL C.A. #31107



13222 3RD STREET EAST MADERIA BEACH, FL

BUILDER

OMAR ABBAS ABBAS DEVELOPMENT **BUILDING CONTRACTOR** 210 S PINELLAS AVE SUITE 220 727-946-0475

DESIGNER

Curtis Morgan

Morgancastle Studio, Inc. Residential Design Services

> 9324 Wildwood Ave. Hudson, FL 34669 Phone: (727)247-8148

morgancastlestudio@gmail.com

REVISIONS

SET SHEET FINAL PERMIT

DATE 11-08-2024

STRUCTURAL WALL SECTION

GENERAL NOTES AND SPECIFICATIONS

2023 FLORIDA BUILDING CODE, RESIDENTIAL 8TH EDITION AMERICAN CONCRETE INSTITUTE OF STRUCTURAL CONCRETE (ACI 318). AMERICAN CONCRETE INSTITUTE OF MASONRY STRUCTURES (TMS 402/ACI 530/ASCE 5) THE MASONRY SOCIETY DIRECT DESIGN HANDBOOK FOR MASONRY STRUCTURES (TMS 403) AMERICAN SOCIETY OF CIVIL ENGINEERS MINIMUM DESIGN LOADS FOR BUILDINGS & OTHER STRUCTURES (ASCE-1-22) SPECIFICATION FOR THE DESIGN FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS LATEST ED DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES BY THE TRUSS PLATE INSTITUTE (TPI) LATEST DESIGN NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) LATEST EDITION.

IT IS THE CONTRACTORS RESPONSIBILITY TO REVIEW ALL DRAWINGS BEFORE CONSTRUCTION BEGINS. THE ENGINEER OF RECORD IS RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS PROJECT ONLY. ANY DISCREPANCY BETWEEN FIELD CONDITIONS, OTHER DESIGN PROFESSIONALS' SHOP DRAWINGS, CONTRACTORS' BUILDING METHODS, AND THESE SIGNED AND SEALED DRAWINGS MUST BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. ENGINEER ASSUMES NO RESPONSIBILITIES FOR ANY EXISTING CONDITIONS (UN.O.)

DESIGN CRITERIA

ROOF LOADS

ROOF LIVE LOADS - 20 PSF ROOF DEAD LOAD (SHINGLE ROOF) - 20 PSF ROOF DEAD LOAD (TILE ROOF) - 25 PSF

ADDITIONAL ATTIC STORAGE LIVE LOAD - 10 PSF

ATTIC SPACE SHALL BE DESIGNED AS UNINHABITABLE FLOOR LOADS

LIVING AREA LIVE LOADS - 40 PSE LIVING AREA DEAD LOADS - 15 PSF (CARPET OR WOOD) LIVING AREA DEAD LOADS - 20 PSF (TILE) GUARD IN-FILL COMPONENTS - 50**

* SINGLE CONCENTRATED LOAD APPLIED AT ANY DIRECTION ALONG THE TOP ** GUARD IN FILL (EXCEPT HANDRAIL) BALUSTERS AND PANEL FILLERS SHALL BE DESIGNED TO WITHSTAND A HORIZONTALLY APPLIED LOAD OF 50 PSF. LOAD NEED NOT BE ASSUMED TO ACT CONCURRENTLY WITH ANY OTHER LIVE LOAD REQUIREMEN

BALCONY LOADS

BALCONY LIVE LOAD - 60 PSF BALCONY DEAD LOAD - 20 PSF (CONTACT ENGINEER FOR ANY FLOORING THAT EXCEEDS

FOUNDATION DESIGN (SHALLOW FOUNDATIONS) ALL FOUNDATIONS ARE CENTERED UNDER SUPPORTED COLUMNS AND WALLS

ALL EXTERIOR WALLS SHALL SUPPORTED BY CONCRETE FOOTERS AS PER FBC 403.1. SEE

TYPICAL WALL SECTION FOR DETAILS. FOOTINGS SHALL BE LEVEL OR SHALL BE STEPPED SO THAT BOTH TOP AND BOTTOM OF SUCH FOOTINGS ARE LEVEL. THE BOTTOM OF ALL FOOTINGS, EXCEPT MONOLITH SLAB-ON-GRADE INTERIOR FOOTINGS, SHALL BE A MINIMUM OF 12 INCHES (305 MM) BELOW

FOUNDATION STEMWALLS SHALL BE AS THICK AS OR THICKER THAN THE WALL SUPPORTED

ABOVE, BUT IN NO CASE LESS THAN 8 INCHES (203 MM) THICK FOR MASONRY

SOIL DESIGN CRITERIA

MINIMUM ALLOWABLE SOIL PRESSURE 2000 PSF.

IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY SOIL CONDITIONS AND REQUIRED COMPACTION

ENGINEER STRONGLY RECOMMENDS A REGISTERED GEOTECHINCAL ENGINEER SHALL VERIFY ACTUAL CONDITIONS PRIOR TO PLACEMENT OF THE FOOTING. IF THE FOUNDATION IS ON PREPARED FILL THE REGISTERED GEOTECHNICAL ENGINEER SHALL VERIFY THE SUITABILITY OF FILL FOR USE AND DETERMINE THE BEARING CAPACITY. ENGINEER IS NOT RESPONSIBLE FOR EXISTING SOIL CONDITIONS AND HAS BASED THE FOUNDATION DESIGN ON A MINIMUM SOIL BEARING CAPACITY OF 2,000 PSF

PRESUMPTIVE CAPACITIES

THE ALLOWABLE BEARING CAPACITIES FOR SOILS CONSISTING OF UNDISTURBED SAND, OR SAND AND ROCK, MAY BE TAKEN AS A MAXIMUM OF 2000 PSF UNLESS A HIGHER VALUE IS SUBSTANTIATED BY RECOGNIZED TEST ANALYSIS OR PROCEDURE. THIS VALUE IS CONSIDERED SAFE WITH RESPECT TO ACTUAL FAILURE OF THE SUPPORTING GROUND, BUT DOES NOT NECESSARILY ENSURE THE PREVENTION OF EXCESSIVE FOUNDATION

REPLACEMENT OF DEFECTIVE SOIL

SOIL UNDER THE FOOTINGS CONTAMINATED WITH ORGANIC MATERIALS OR TRASH MUST BE REPLACED WITH CLEAN FILL AS BELOW IF THERE IS ANY EVIDENCE THAT THE SOILS ARE NOT UNDISTURBED SAND OR SAND AND ROCK.

FILL TO SUPPORT FOUNDATIONS

A FILLED SUBGRADE LAID IN 6 INCH LIFTS, SHALL BE THOROUGHLY COMPACTED BY APPROVED METHODS. ALL FILL PLACED UNDER FOUNDATIONS SHALL BE CLEAN SAND OR ROCK, FREE OF DEBRIS AND OTHER DELETERIOUS MATERIALS, VOIDS AROUND ROCKS SHALL BE CAREFULLY FILLED AND PROPERLY

THE FILL SHALL BE COMPACTED TO A MINIMUM OF 95% OF MAXIMUM DRY DENSITY FOR ALL LAYERS, AS VERIFIED

TERMITE PROTECTION

SECTION R318 PROTECTION AGAINST TERMITES

R3I8.I TERMITE PROTECTION. TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTATIVE TREATMENT TO NEW CONSTRUCTION. SEE SECTION 202, "REGISTERED TERMITICIDE." UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

R318.1.1 IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, THE INITIAL CHEMICAL SOIL TREATMENT INSIDE THE FOUNDATION PERIMETER SHALL BE DONE AFTER ALL EXCAVATION, BACKFILLING AND COMPACTION IS COMPLETE. R318.12 IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, SOIL AREA DISTURBED AFTER INITIAL CHEMICAL SOIL TREATMENT SHALL BE RETREATED WITH A CHEMICAL SOIL TREATMENT, INCLUDING SPACES BOXED OR FORMED. R3I8.13 IF 60IL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, SPACE IN CONCRETE FLOORS BOXED OUT OR FORMED FOR THE SUBSEQUENT INSTALLATION OF PLUMBING TRAPS, DRAINS OR ANY OTHER PURPOSE SHALL BE CREATED BY USING PLASTIC OR METAL PERMANENTLY PLACED FORMS OF SUFFICIENT DEPTH TO ELIMINATE ANY PLANNED SOIL DISTURBANCE AFTER INITIAL CHEMICAL SOIL TREATMENT. R318.1.4 IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, CHEMICALLY TREATED SOIL SHALL BE

PROTECTED WITH A MINIMUM 6 MIL VAPOR RETARDER TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFORE VAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED. ANY WORK, INCLUDING PLACEMENT OF REINFORCING STEEL, DONE AFTER CHEMICAL TREATMENT UNTIL THE CONCRETE FLOOR IS POURED, SHALL BE DONE IN SUCH MANNER AS TO AVOID PENETRATING OR DISTURBING TREATED SOIL. R318.15 IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, CONCRETE OVERPOUR OR MORTAR

ACCUMULATED ALONG THE EXTERIOR FOUNDATION PERIMETER SHALL BE REMOVED PRIOR TO EXTERIOR CHEMICAL SOIL TREATMENT, TO ENHANCE VERTICAL PENETRATION OF THE CHEMICALS. R318.16 IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, CHEMICAL SOIL TREATMENTS SHALL ALSO BE APPLIED UNDER ALL EXTERIOR CONCRETE OR GRADE WITHIN I FOOT OF THE PRIMARY STRUCTURE SIDEWALLS, ALSO, A VERTICAL CHEMICAL BARRIER SHALL BE APPLIED PROMPTLY AFTER CONSTRUCTION IS COMPLETED, INCLUDING INITIAL LANDSCAPING AND IRRIGATION/SPRINKLER INSTALLATION. ANY SOIL DISTURBED AFTER THE CHEMICAL VERTICAL BARRIER IS APPLIED SHALL BE PROMPTLY RETREATED.
R318.1.7 IF A REGISTERED TERMITICIDE FORMULATED AND REGISTERED AS A BAIT SYSTEM IS USED FOR SUBTERRANEAN TERMITE

SECTIONS R318.1.1 THROUGH R318.1.6 DO NOT APPLY & HOWEVER, A SIGNED CONTRACT ASSURING THE INSTALLATION, MAINTENANCE AND MONITORING OF THE BAITING SYSTEM THAT IS IN COMPLIANCE WITH THE REQUIREMENTS OF CHAPTER 482, FLORIDA STATUTES SHALL BE PROVIDED TO THE BUILDING OFFICIAL PRIOR TO THE POURING OF THE SLAB, AND THE SYSTEM MUST BE INSTALLED PRIOR TO FINAL BUILDING APPROVAL. IF THE BAITING SYSTEM DIRECTIONS FOR USE REQUIRE A MONITORING PHASE PRIOR TO INSTALLATION OF THE PESTICIDE ACTIVE INGREDIENT, THE INSTALLATION OF THE MONITORING

PHASE COMPONENTS SHALL BE DEEMED TO CONSTITUTE INSTALLATION OF THE SYSTEM.
R318.1.8 IF A REGISTERED TERMITICIDE FORMULATED AND REGISTERED AS A WOOD TREATMENT IS USED FOR SUBTERRANEAN
TERMITE PREVENTION, SECTIONS R318.1.1 THROUGH R318.1.6 DO NOT APPLY. APPLICATION OF THE WOOD TREATMENT TERMITICIDE SHALL BE AS REQUIRED BY LABEL DIRECTIONS FOR USE, AND MUST BE COMPLETED PRIOR TO FINAL BUILDING APPROVAL.

MASONRY NOTES

MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF SPECIFICATION FOR MASONRY STRUCTURES (ACI 530/ASCE 6/TMS 602)". PUBLISHED BY THE MASONRY SOCIETY, BOULDER, COLORADO THE AMERICAN CONCRETE INSTITUTE, FARMINGTON HILLS, MICHIGAN: AND THE AMERICAN SOCIETY OF CIVIL ENGINEERS RESTON, VIRGINIA: EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THESE CONTRACT DOCUMENTS. TESTING OF FIELD MATERIALS FOR QUALITY CONTROL IS NOT REQUIRED BY ENGINEER FOR THIS PROJECT. COMPRESSIVE STRENGTH REQUIREMENT IS 1'm=1500 PSI. DETERMINATION OF COMPRESSIVE STRENGTH IS THE ALLOWABLE STRESS METHOD. UNIT STRENGTH METHOD IS NOT APPLICABLE. QUALITY STRENGTH METHOD IS NOT APPLICABLE. GROUT SHALL COMPLY WITH ASTM C416. GROUT SHALL BE 3000 PSI UNO, AND HAYE A SLUMP RANGE OF 8'-11'. MORTAR MATERIALS SHALL BE TYPE M OR S GRAY MORTAR MASONRY UNIT MATERIALS SHALL BE 1900 PSI MIN. CONCRETE MASONRY UNIT. REINFORCEMENT, PRE-STRESSED TENDONS AND METAL ACCESSORIES SHALL BE 60 KSI REBAR (MIN.). WELDED WIRE

FABRIC TO BE INSTALLED AS SPECIFIED ON PLAN SET. STAINLESS STEEL IS NOT APPLICABLE. COATING FOR CORROSION PROTECTION IS NOT APPLICABLE. CORROSION PROTECTION FOR TENDONS IS NOT APPLICABLE. PRE-STRESSING ANCHORAGE, COUPLERS AND END BLOCKS ARE NOT APPLICABLE. JOINT FILLERS ARE NOT APPLICABLE.

LINTELS TO BE BY CAST-CRETE UNLESS OTHERWISE NOTED.

CONCRETE FOOTERS & GRADE BEAMS - 3000 PSI MIN CAST CRETE LINTELS - 3000 PSI MIN GROUTED MASONRY - 3000 PSI MIN

REQUIRED SAFETY GLAZING IN HAZARDOUS LOCATIONS

THE FOLLOWING SHALL BE CONSIDERED SPECIFIC HAZARDOUS LOCATIONS FOR THE PUROPSE OF GLAZING:

GLAZING IN SWINGING DOORS AND FIXED GLASS GLASS AND SLIDING GLASS DOORS (PANELS) DOOR ASSEMBLIES. GLAZING IN DOORS AND ENCLOSURES FOR HOT TUBS, WHIRLPOOLS, SAUNAS, STEAM ROOMS BATHTUBS, AND SHOWERS. GLAZING IN ANY PORTION OF A BUILDING WALL ENCLOSING THESE COMPARTMENTS WHERE THE BOTTOM EDGE OF THE GLAZING IS LESS THAN 60° (1524 MM) ABOVE THE FLOOR OR

B. GLAZING IN AN INDIVIDUAL FIXED OR OPERABLE PANEL ADJACENT A DOOR WHERE THE NEAREST VERTICAL EDGE 15 WITHIN A 24-INCH (610 MM) RADIUS OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS LESS THAN 60 INCHES (1524 MM) ABOVE THE FLOOR OR WALKING

4. GLAZING IN AN INDIVIDUAL FIXED OR OPERABLE PANEL OTHER THAN THOSE LOCATIONS DESCRIBED IN ITEMS 2 AND 3 ABOVE, THAT MEETS ALL OF THE

FOLLOWING CONDITIONS: EXPOSED AREA OF AN INDIVIDUAL PANE GREATER THAN 9.0 (.84 M SQ.). BOTTOM EDGE LESS THAN 18 INCHES (451 MM) ABOVE THE FLOOR

TOP EDGE GREATER THAN 36 INCHES (914 MM) ABOVE THE FLOOR. ONE OR MORE WALKING SURFACES WITHIN 36 INCHES (914 MM) HORIZONTALLY OF THE PANE OF GLAZING. 5. ALL GLAZING IN RAILING\$ REGARDLE\$5 OF AREA OR HEIGHT ABOYE A WALKING \$URFACE INCLUDING \$TRUCTURAL BALU\$TER PANEL\$ AND

6. GLAZING IN WALLS AND FENCES ENCLOSING INDOOR AND OUTDOOR SWIMMING POOLS WHERE THE BOTTOM EDGE OF THE GLAZING IS (1) LESS THAN 600 INCHES (1525 MM) ABOVE THE WALKING SURFACE ON THE POOL SIDE, AND (2) WITHIN 36 INCHES (914 MM) HORIZONTALLY OF THE WALKING SURFACE ON THE POOL SIDE THIS SHALL APPLY TO SINGLE GLAZING AND ALL PANES IN MILLTIPLE GLAZING ADJACENT TO THE BOTTOM STAIR LANDING GLAZING ADJACENT TO THE LANDING AT THE BOTTOM OF A STAIRWAY WHERE THE GLAZING IS LESS THAN 36 INCHES (914 MM) ABOVE THE LANDING AND WITHIN A 60-INCH (1524 MM) HORIZONTAL ARC LESS THAN 180 DEGREES FROM THE BOTTOM TREAD NOSING SHALL BE CONSIDERED TO BE A HAZARDOUS

AIR HANDLER GENERAL NOTES

R40336 AIR-HANDLING UNITS AIR-HANDLING UNITS SHALL NOT BE INSTALLED IN THE ATTIC WHEN A HOME IS BROUGHT INTO CODE COMPLIANCE BY SECTION R402, AIR-HANDLING UNITS SHALL BE ALLOWED IN ATTICS FOR COMPLIANCE BY SECTION R405 ONLY IF THE FOLLOWING CONDITIONS ARE MET:

THE SERVICE PANEL OF THE EQUIPMENT IS LOCATED WITHIN 6 FEET (1829 MM) OF AN ATTIC ACCESS. A DEVICE IS INSTALLED TO ALERT THE OWNER OR SHUT DOWN THE UNIT WHEN THE CONDENSATION DRAIN

IS NOT WORKING PROPERLY. THE ATTIC ACCESS OPENING IS OF SUFFICIENT SIZE TO REPLACE THE AIR HANDLER. A NOTICE IS POSTED ON THE ELECTRIC SERVICE PANEL INDICATING TO THE HOMEOWNER THAT THE AIR HANDLER IS LOCATED IN THE ATTIC. SAID NOTICE SHALL BE IN ALL CAPITALS, IN 16-POINT TYPE, WITH THE TITLE AND FIRST PARAGRAPH IN BOLD: NOTICE TO HOMEOWNER A PART OF YOUR AIR-CONDITIONING

SYSTEM, THE AIR HANDLER, IS LOCATED IN THE ATTIC. FOR PROPER, EFFICIENT AND ECONOMIC

OPERATION OF THE AIR CONDITIONING SYSTEM, YOU MUST ENSURE THAT REGULAR MAINTENANCE IS PERFORMED. YOUR AIR-CONDITIONING SYSTEM IS EQUIPPED WITH ONE OR BOTH OF THE FOLLOWING: (1) A DEVICE THAT WILL ALERT YOU WHEN THE CONDENSATION DRAIN IS NOT WORKING PROPERLY OR (2) A DEVICE THAT WILL SHUT DOWN THE SYSTEM WHEN THE CONDENSATION DRAIN IS NOT WORKING. TO LIMIT POTENTIAL DAMAGE TO YOUR HOME, AND TO AVOID DISRUPTION OF SERVICE, IT IS RECOMMENDED THAT YOU ENSURE PROPER WORKING ORDER OF THESE DEVICES BEFORE EACH SEASON OF PEAK OPERATION.

SPECIFICATIONS FOR LANDINGS AND STAIR CONSTRUCTION

* ALL STRINGERS TO BE (2) 2x12 NO. 2 5.Y.P.

* ALL STRINGERS TO ATTACH TO LANDINGS AND UPPER FLOOR LEVELS WITH SIMPSON LSC HANGER. STRINGERS RUNNING PARALLEL TO AND SUPPORTED BY A WALL SHALL BE ATTACHED: AT MASONRY WITH 1/4" DIA x 3" TAPCONS AT 8" O.C. STAGGERED ALONG THE LENGTH OF OF THE STRINGER (USE PT AT MAS/CONC.). AT WOOD WITH (4) Ø.131 x 3-1/2" NAILS AT EACH STUD MIN. 16" O.C. IF STRINGERS ARE SUPPORTED IN THIS WAY, LSU HANGERS ARE NOT REQUIRED AT THE ENDS.

• ALL STRINGERS SPRINGING FROM A FLAT SURFACE SHALL TERMINATE AT 2x4 FOOT (TO BE PT AT CONC.) ATTACH FOOT TO FLOOR: AT CONC. WITH 1/4" DIA x 3" APCONS AT 12" O.C., AT WOOD ATTACH Ø.131 x 3-1/2" NAILS AT 8" O.C., TO TRUSS/JOIST OR BLOCKING, BELOW, ATTACH STRINGER TO FOOT WITH (2) Ø.131 x 3-1/2" TOENAILS EA. LANDINGS SHALL BE BUILT AS FOLLOWS: + JOIST TO BE 2x8 NO.2 S.Y.P. MIN 16" O.C. • IF LANDING INCLUDES STRINGER SUPPORT IN A BEAM CONFIGURATION, BEAM SHALL BE (2) 2x12 NO2 S.Y.P. WITH (2) 2x4 SUPPORT AT EACH END. • IF LANDING IS SUPPORTED BY A KNEE WALL, PROVIDE A 2x8 END JOIST WITH (3) Ø.131 x 3-1/21 R.S. END NAILS AT EACH LANDING JOIST, AND (2) Ø.131 x 3-1/21 TOE NAILS FROM EACH JOIST TO KNEE WALL TOP PLATE.

LEDGERS TO BE 2x8 WITH: * (4) \emptyset .131 \times 3-1/2" NAILS AT EACH PASSING STUD AT WOOD ATTACHMENT 16" O.C. MAX. * 1/4" x 3" TAPCONS AT 8" O.C. STAGGERED AT MASONRY/CONCRETE ATTACHMENT

· ATTACH JOIST TO LEDGERS OR BEAMS WITH SIMPSON LUS26 HANGERS OR EQUAL. THE SPECIFICATION LISTED ABOVE OR MINIMUM SPECS. SUBSTITUTIONS OF EQUAL OR BETTER CAPACITY ARE ACCEPTABLE BUARDRAILS AND HANDRAILS ARE BY OTHERS, INCLUDING THEIR ATTACHMENTS. ALL RAIL SYSTEMS MUST MEET OR EXCEED THE REQUIREMENTS OF FBC TABLE R3Ø15 AND SECTION R312.

FLASHING REQUIREMENTS

FLASHING REQUIREMENTS DUE TO CLARITY NOT ALL REQUIRED FLASHING IS INDICATED ON THE DRAWINGS. FLASHING SHALL BE INSTALLED PER FBC 2023 RTI03.4. CODE SECTION HAS BEEN PROVIDED BELOW AS REFERENCE ONLY RTI03.4 FLASHING. APPROVED CORROSION-RESISTANT FLASHING SHALL BE APPLIED SHINGLE-FASHION IN A MANNER TO PREVENT ENTRY OF WATER INTO THE WALL CAVITY OR PENETRATION OF WATER TO THE BUILDING STRUCTURAL FRAMING COMPONENTS, SELF-ADHERED MEMBRANES USED AS FLASHING SHALL COMPLY WITH AAMA 111. ALL EXTERIOR ENESTRATION PRODUCTS SHALL BE SEALED AT THE JUNCTURE WITH THE BUILDING WALL WITH A SEALANT COMPLYING WITH AAMA 800 OR ASTM C920 CLASS 25 GRADE NS OR GREATER FOR PROPER JOINT EXPANSION AND CONTRACTION ASTM CIZSI, AAMA SIZ, OR OTHER APPROVED STANDARD AS APPROPRIATE FOR THE TYPE OF SEALANT, FLUID-APPLIED MEMBRANES USED AS FLASHING IN EXTERIOR WALLS SHALL COMPLY WITH AAMA 114. THE FLASHING SHALL EXTEND TO HE SURFACE OF THE EXTERIOR WALL FINISH. APPROVED CORROSION-RESISTANT FLASHING SHALL BE INSTALLED AT

HE FOLLOWING LOCATIONS EXTERIOR WINDOW AND DOOR OPENINGS, FLASHING AT EXTERIOR WINDOW AND DOOR OPENINGS SHALL EXTEND TO THE SURFACE OF THE EXTERIOR WALL FINISH OR TO THE WATER-RESISTIVE BARRIER COMPLYING WITH SECTION 1032 FOR SUBSEQUENT DRAINAGE. MECHANICALLY ATTACHED FLEXIBLE FLASHINGS SHALL COMPLY WITH AAMA 112.

OF THE FOLLOWING: THE FENESTRATION MANUFACTURER 'S INSTALLATION AND FLASHING INSTRUCTIONS, OR FOR APPLICATIONS NOT ADDRESSED IN THE FENESTRATION MANUFACTURER S INSTRUCTIONS, IN ACCORDANCE WITH THE FLASHING MANUFACTURER 'S INSTRUCTIONS. WHERE FLASHING INSTRUCTIONS OR DETAILS ARE NOT PROVIDED, PAN FLASHING SHALL BE INSTALLED AT THE SILL OF EXTERIOR WINDOW AND DOOR OPENINGS, PAN FLASHING SHALL BE SEALED OR SLOPED IN SUCH A MANNER AS TO DIRECT WATER TO THE SURFACE OF THE EXTERIOR WALL TNISH OR TO THE WATER-RESISTIVE BARRIER FOR SUBSEQUENT DRAINAGE. OPENINGS USING PAN FLASHING

SHALL INCORPORATE FLASHING OR PROTECTION AT THE HEAD AND SIDES. IN ACCORDANCE WITH THE FLASHING DESIGN OR METHOD OF A REGISTERED DESIGN PROFESSIONAL. IN ACCORDANCE WITH OTHER APPROVED METHODS.
IN ACCORDANCE WITH FMA/AAMA 100, FMA/AAMA 200, FMA/WIDMA 250, FMA/AAMA/ WIDMA 300 OR

FMA/AAMA/WDMA 400.

AT THE INTERSECTION OF CHIMNEYS OR OTHER MASONRY CONSTRUCTION WITH FRAME OR STUCCO WALLS, WITH PROJECTING LIPS ON BOTH SIDES UNDER STUCCO COPINGS.
UNDER AND AT THE ENDS OF MASONRY, WOOD OR METAL COPINGS AND SILLS

CONTINUOUSLY ABOVE ALL PROJECTING WOOD TRIM. WHERE EXTERIOR PORCHES, DECKS OR STAIRS ATTACH TO A WALL OR FLOOR ASSEMBLY OF WOOD-FRAME

CONSTRUCTION.
AT WALL AND ROOF INTERSECTIONS.

AT BUILT-IN GUTTERS

WALL COVERING NOTES

RT036, LATH, ALL LATH AND LATH ATTACHMENTS SHALL BE OF CORROSION-RESISTANT MATERIALS. EXPANDED METAL OR WOVEN WIRE LATH SHALL BE ATTACHED WITH 11/2-INCH-LONG (38 MM), 11 GAGE NAILS HAVING A 7/16-INCH (II.I MM) HEAD, OR 7/8-INCHLONG (222 MM), IG GAGE STAPLES, SPACED AT NO MORE THAN 6 INCHES (152 MM), OR AS OTHERWISE APPROVED.

R103.6.2 PLASTER PLASTERING WITH PORTLAND CEMENT PLASTER SHALL BE NOT LESS THAN THREE COATS WHEN APPLIED OVER METAL LATH OR WIRE LATH AND SHALL BE NOT LESS THAN TWO COATS WHEN APPLIED OVER MASONRY, CONCRETE, PRESSURE-PRESERVATIVE TREATED WOOD OR DECAY-RESISTANT WOOD AS SPECIFIED IN SECTION

IF THE PLASTER SURFACE IS COMPLETELY COVERED BY VENEER OR OTHER FACING MATERIAL OR IS COMPLETELY CONCEALED, PLASTER APPLICATION NEED BE ONLY TWO COATS, PROVIDED THE TOTAL THICKNESS IS AS SET FORTH IN TABLE RT02.1(1).

R70362.1 WEEP SCREEDS. A MINIMUM 0019-INCH (05 (NO. 26 GALVANIZED SHEET GAGE), CORROSION-RESISTANT WEEP SCREED OR PLASTIC WEEP SCREED, WITH A MINIMUM VERTICAL ATTACHMENT FLANGE OF 31/2 INCHES (89 MM) SHALL BE PROVIDED AT OR BELOW THE FOUNDATION PLATE LINE ON EXTERIOR STUD WALLS IN ACCORDANCE WITH ASTM C 926. THE WEEP SCREED SHALL BE PLACED A MINIMUM OF 4 INCHES (102 MM) ABOVE THE EARTH OR 2 INCHES (51 MM) ABOVE PAVED AREAS AND SHALL BE OF A TYPE THAT WILL ALLOW TRAPPED WATER TO DRAIN TO THE EXTERIOR OF THE BUILDING. THE WEATHER-RESISTANT BARRIER SHALL LAP THE ATTACHMENT FLANGE. THE EXTERIOR LATH SHALL COVER AND TERMINATE ON THE ATTACHMENT FLANGE OF

R10363 WATER-RESISTIVE BARRIERS. WATER-RESISTIVE BARRIERS SHALL BE INSTALLED AS REQUIRED IN SECTION RT03.2 AND, WHERE APPLIED OVER WOOD-BASED SHEATHING, SHALL INCLUDE A WATER-RESISTIVE VAPOR-PERMEABLE BARRIER WITH A PERFORMANCE AT LEAST EQUIVALENT TO TWO LATERS OF GRADE D PAPER. THE INDIVIDUAL LAYERS SHALL BE INSTALLED INDEPENDENTLY SUCH THAT EACH LAYER PROVIDES A SEPARATE CONTINUOUS PLANE AND ANY FLASHING (INSTALLED IN ACCORDANCE WITH SECTION RT03.8) INTENDED TO DRAIN TO THE WATER-RESISTIVE BARRIER IS DIRECTED BETWEEN

EXCEPTION: WHERE THE WATER-RESISTIVE BARRIER THAT IS APPLIED OVER WOOD-BASED SHEATHING HAS A WATER RESISTANCE EQUAL TO OR GREATER THAN THAT OF 60-MINUTE GRADE D PAPER AND IS SEPARATED FROM THE STUCCO BY AN INTERVENING, SUBSTANTIALLY NONWATER-ABSORBING LAYER OR DESIGNED DRAINAGE SPACE

FRAMING NOTES

MINIMUM OF 3-PLY 2x STUD 5.Y.P. COLUMNS TO BE INSTALLED AT BEAM OR GIRDER TRUSS BEARING LOCATIONS UN.O.

MINIMUM OF 3-PLY 2x STUD 5,Y.P. COLUMNS TO BE INSTALLED AT BEAM OR GIRDER TRUSS BEARING LOCATIONS UNO ALL TRUSS BRACING LUMBER SHALL BE S.Y.P. NO.2 OR BETTER.

INTERIOR LOAD BEARING (IF APPLICABLE) WALL STUDS SPACED AT 16" O.C. AND SHALL BE S.Y.P. NO.2 OR BETTER.

TYPICAL AT ALL LOAD BEARING S.Y.P. COMPONENTS, NO.1 GRADE SHALL BE USED FOR 2x4, FOR 2x DEPTHS GREATER THAN 2x4, NO.2 GRADE OR BETTER MAY BE USED. ALL 4x MATERIAL MAY BE NO.2 GRADE UN.O.

INTERIOR NON-LOAD BEARING WALLS SHALL BE UTILITY GRADE OR BETTER

INSTALL BLOCKING IN ALL WALL STUDS OVER 8'-0" AT MID-HEIGHT, AND SHEATHING JOINT.

BRACE GABLE END WALLS AT 4'-0" O.C. AS SHOWN IN DRAWINGS

ALL LOAD BEARING WALLS SHALL HAVE S.Y.P. DOUBLE TOP PLATES AND SHALL BE FASTENED PER P/S5. BOTTOM PLATES SHALL BE S.Y.P. (PT AT CONC.)

INTERIOR BEARING COLUMNS TO HAVE S.Y.P. TOP PLATE.

COLUMNS WITH REACTION LOADS OVER 5000* TO BE CONTINUOUS THRU TOP PLATE TO GIRDER ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED OR NATURAL DURABLE WOOD.

PRESSURE TREATED LUMBER SHALL BE IMPREGNATED WITH CCA SALT TREATMENT IN ACCORDANCE WITH F.S. II-W-57I AND BARE THE AMERICAN WOOD PRESERVES INSTITUTE EQUALITY MARK LP-2. ALL SHEATHING TO BE SPAN RATED FOR APPROPRIATE APPLICATION.

ALL ROOF SHEATHING ATTACHMENT, CLIPS (MAX. 24" O.C.) REFER TO PLANS FOR SHEATHING THICKNESS, ATTACHMENT.

ALL NAILING AND BOLTING SHALL COMPLY WITH AMERICAN INSTITUTE OF TIMBER CONSTRUCTION REQUIREMENTS, ALL NAILS EXPOSED TO THE EXTERIOR SHALL BE GALVANIZED

ALL CONNECTION HARDWARE SHALL BE GALVANIZED AND SUPPLIED BY SIMPSON STRONG TIE CO., USP OR EQUIVALENT. SUBMIT CUT SHEETS FOR ALL CONNECTION HARDWARE TO ENGINEER FOR APPROVAL. ALL NAIL HOLES SHALL BE FILLED OR AS RECOMMENDED BY THE MANUFACTURER.

BRACING: TEMPORARY BRACING OF THE ROOF SYSTEM SHALL BE INSTALLED PER BCSI RECOMMENDATIONS AND SHALL BE UTILIZED AS THE PERMANENT BRACING FOR THE ROOF SYSTEM. ALL WOOD FRAMING SHALL BE IN COMPLIANCE WITH THE LATEST NDS EDITION FOR WOOD CONSTRUCTION.

WOOD DESIGN CRITERIA

NAILING SCHEDULE.

WOOD FRAMING MEMBERS SHALL BE 2 SOUTHERN YELLOW PINE (UNO.) WITH AN ALLOWABLE BENDING STRESS (Fb) = 1000 PSI AND A MODULUS OF ELASTICITY = 1,400,000 PSI (DOES NOT INCLUDE INTERIOR NON LOAD BEARING STUD WALLS)

DESIGN, FABRICATE AND ERECT WOOD TRUSSES IN ACCORDANCE WITH THE DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES BY THE TRUSS PLATE INSTITUTE.

ALL EXPOSED WOOD OR WOOD IN CONTACT WITH EARTH OR CONCRETE SHALL BE PRESSURE TREATED

UNTREATED WOOD SHALL NOT BE IN DIRECT CONTACT WITH CONCRETE. SEAT PLATES SHALL BE PROVIDED AT BEARING

GENERAL TRUSS ENGINEERING NOTES

SHALL BE DESIGNED IN ACCORDANCE WITH APPROVED ENGINEERING PRACTICE. THE DESIGN AND MANUFACTURE OF METAL PLATE CONNECTED WOOD TRUSSES SHALL COMPLY WITH ANSI/TPI I. THE TRUSS DESIGN DRAWINGS SHALL BE PREPARED BY A REGISTERED PROFESSIONAL WHERE REQUIRED BY FLORIDA STATUTES

R502.1.102 3/LIRC 502.1123/8 BRACING. TRUGGES SHALL BE BRACED TO PREVENT ROTATION AND PROVIDE LATERAL STABILITY IN ACCORDANCE WITH THE REQUIREMENTS SPECIFIED IN THE CONSTRUCTION DOCUMENTS FOR THE BUILDING AND ON THE INDIVIDUAL TRUSS DESIGN DRAWINGS. IN THE ABSENCE OF SPECIFIC BRACING REQUIREMENTS, TRUSSES SHALL BE

TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, SPLICED OR OTHERWISE ALTERED IN ANY WAY WITHOUT THE APPROVAL OF A REGISTERED DESIG PROFESSIONAL. ALTERATIONS RESULTING IN THE ADDITION OF LOAD (E.G., HVAC EQUIPMENT, WATER HEATER, ETC.), THAT EXCEED THE DESIGN LOAD FOR THE TRUSS, SHALL NOT BE PERMITTED WITHOUT VERIFICATION THAT THE TRUSS IS CAPABLE OF SUPPORTING THE ADDITIONAL LOADING.

R502.1.10.4 34 IRC 502.11.47% TRUSS DESIGN DRAWINGS. TRUSS DESIGN DRAWINGS, PREPARED IN COMPLIANCE WITH SECTION R502.1.10.1, SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO INSTALLATION. TRUSS DESIGN DRAWINGS SHALL BE PROVIDED WITH THE SHIPMENT OF TRUSSES DELIVERED TO THE JOB SITE. TRUSS DESIGN DRAWINGS SHALL INCLUDE, AT A MINIMUM, THE INFORMATION SPECIFIED BELOW:

1. SLOPE OR DEPTH, SPAN AND SPACING.

R502.1.10.3 34 IRC 502.11.3 % ALTERATIONS TO TRUSSES.

2. LOCATION OF ALL JOINTS. 3. REQUIRED BEARING WIDTHS

4 DESIGN LOADS AS APPLICABLE

4.I. TOP CHORD LIVE LOAD ± 42. TOP CHORD DEAD LOAD:

3. BOTTOM CHORD LIVE LOAD! 4.4. BOTTOM CHORD DEAD LOAD\$
4.5. CONCENTRATED LOAD\$ AND THEIR POINTS OF APPLICATION\$ AND

4.6. CONTROLLING WIND AND EARTHQUAKE LOADS.

5. ADJUSTMENTS TO LUMBER AND JOINT CONNECTOR DESIGN VALUES FOR CONDITIONS OF USE. 6. EACH REACTION FORCE AND DIRECTION.

1. JOINT CONNECTOR TYPE AND DESCRIPTION, E.G., SIZE, THICKNESS OR GAUGE, AND THE DIMENSIONED LOCATION OF EACH JOINT CONNECTOR EXCEPT WHERE SYMMETRICALLY

LOCATED RELATIVE TO THE JOINT INTERFACE. 8. LUMBER SIZE, SPECIES AND GRADE FOR EACH MEMBER

9. CONNECTION REQUIREMENTS FOR: 9.1. TRUSS-TO-GIRDER-TRUSS:

9.2. TRUSS PLY-TO-PLY AND

9.3. FIELD SPLICES. IØ. CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DESCRIPTION FOR LIVE AND TOTAL LOAD.

IN MAXIMUM AXIAL COMPRESSION FORCES IN THE TRUSS MEMBERS TO ENABLE THE BUILDING DESIGNER TO DESIGN THE SIZE, CONNECTIONS AND ANCHORAGE OF THE PERMANENT CONTINUOUS LATERAL BRACING. FORCES SHALL BE SHOWN ON THE TRUSS DRAWING OR ON SUPPLEMENTAL DOCUMENTS. 12. REQUIRED PERMANENT TRUSS MEMBER BRACING LOCATION.

TRUSS DESIGN DRAWINGS, PREPARED IN CONFORMANCE TO SECTION RS02.1.T.I, SHALL BE PROVIDED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO INSTALLATION. TRUSS DESIGN DRAWINGS SHALL INCLUDE, AT A MINIMUM, THE INFORMATION SPECIFIED BELOW. TRUSS DESIGN DRAWINGS SHALL BE PROVIDED WITH THE SHIPMENT OF TRUSSES

ULTIMATE DESIGN WIND SPEED, VULT, AND EXPOSURE CATEGORY.

SLOPE OR DEPTH, SPAN AND SPACING. LOCATION OF ALL JOINTS.

REQUIRED BEARING WIDTH

DESIGN LOADS AS APPLICABLE. 5.1. TOP CHORD LIVE LOAD (AS DETERMINED FROM SECTION R3016). 52. TOP CHORD DEAD LOAD. 5.3. BOTTOM CHORD LIVE LOAD. 5.4. BOTTOM CHORD DEAD LOAD. 5.5. CONCENTRATED LOADS AND THEIR POINTS OF APPLICATION. 5.6. CONTROLLING WIND AND EARTHQUAKE LOADS.

ADJUSTMENTS TO LUMBER AND JOINT CONNECTOR DESIGN VALUES FOR CONDITIONS OF USE. EACH REACTION FORCE AND DIRECTION.

3. JOINT CONNECTOR TYPE AND DESCRIPTION (E.G., SIZE, THICKNESS OR GAGE) AND THE DIMENSIONED LOCATION OF EACH JOINT CONNECTOR EXCEPT WHERE SYMMETRICALLY LOCATED RELATIVE TO THE JOINT INTERFACE.

9. LUMBER SIZE, SPECIES AND GRADE FOR EACH MEMBER.

CONNECTION REQUIREMENTS FOR: 101. TRUSS TO GIRDER-TRUSS. 102. TRUSS PLY TO PLY. 103. FIELD SPLICES. CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DESCRIPTION FOR LIVE AND TOTAL LOAD.

MAXIMUM AXIAL COMPRESSION FORCES IN THE TRUSS MEMBERS TO ENABLE THE BUILDING DESIGNER TO DESIGN

THE SIZE. CONNECTIONS AND ANCHORAGE OF THE PERMANENT CONTINUOUS LATERAL BRACING. FORCES SHALL BE SHOWN ON THE TRUSS DESIGN DRAWING OR ON SUPPLEMENTAL DOCUMENTS.

PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER (DELEGATED ENGINEER) AND FABRICATED IN ACCORDANCE WITH THE NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION OF THE TRUSS PLATE INSTITUTE (TPI). THE TRUSS SYSTEM DESIGNER (DELEGATED ENGINEER) SHALL PREPARE THE TRUSS SYSTEM SHOP DRAWINGS, SUCH SHOP DRAWINGS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL FOR REVIEW AND APPROVAL. THE SHOP DRAWINGS SHALL MEET THE FOLLOWING REQUIREMENTS

I. ALL SHOP DRAWINGS SHALL BE IN CONFORMITY WITH THE ARCHITECT OR ENGINEER OF RECORD FRAMING. PLANS UNLESS PRIOR WRITTEN APPROVAL IS OBTAINED FROM THE ARCHITECT OR ENGINEER OF RECORD. IF REFRAMING IS APPROVED, THE ARCHITECT OR ENGINEER OF RECORD SHALL RESUBMIT REVISED FRAMING PLANS TO THE BUILDING OFFICIAL AFTER RECEIVING UPDATED PLANS FROM THE DELEGATED ENGINEER SHOWING ALL ADJUSTMENTS NECESSARY TO SAFELY TRANSMIT ALL APPLIED LOADS TO THE

2. PERMANENT BRACING OF INDIVIDUAL TRUSS MEMBERS MAY BE REQUIRED ON CERTAIN MEMBERS OF THE TRUSSES TO PREVENT THE MEMBERS FROM BUCKLING IN THE PLANE NORMAL TO THE TRUSSES (BUCKLING IN THE NARROW DIRECTION). THIS BRACING SHALL BE DESIGNED FOR BOTH UPWARD AND DOWNWARD LOADS AND SHALL BE SHOWN ON THE INDIVIDUAL TRUSS DRAWINGS (TRUSS ENGINEERING USUALLY SHOUN ON 81/2-INCH BY 11-INCH (216 MM BY 279 MM) SHEETS ("A" SIZE DRAWINGS), THE DESIGN OF THIS BRACING SHALL BE THE RESPONSIBILITY OF THE DELEGATED ENGINEER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SEEING THAT THIS BRACING IS PROPERLY INSTALLED. THIS BRACING MAY BE IN THE FORM OF (BUT NOT LIMITED) TO "T" BRACING OF AN INDIVIDUAL MEMBER, OR LATERAL BRACING OF A SERIES OF MEMBERS COMMON TO A NUMBER OF TRUSSES, WHERE LATERAL BRACING IS USED, THIS BRACING SHALL BE RESTRAINED AGAINST LATERAL MOVEMENT, IN ACCORDANCE WITH DETAILS PROVIDED BY THE DELEGATED ENGINEER ALL DETAILS AND SECTIONS REQUIRED TO SHOW THE SIZE AND CONNECTIONS OF ALL SECONDARY MEMBERS WILL BE SUPPLIED ON THE DELEGATED ENGINEERING PLANS AND SHALL SHOW ALL FRAMING, CONNECTIONS AND BRACING ON ONE OR MORE PRIMARY PLANS OF MINIMUM SIZE 24 INCHES BY 36 INCHES.

3. A SIZE 8 1/2-INCHES BY 11-INCHES CUT SHEETS SHOWING INDIVIDUAL MEMBER DESIGN SHALL ALSO BE FURNISHED TO THE ENGINEER OF RECORD.

4. THE SIZE AND LOCATION OF ALL PLATES AT EACH JOINT SHALL BE SHOWN ON THE TRUSS DESIGN DRAWINGS.

5. THE CONNECTION BETWEEN TRUSSES SHALL BE DETAILED IN THE SHOP DRAWINGS

6. TRUSS DESIGN DRAWINGS SHALL INDICATE THE SUPPORT AND MINIMUM BEARING OF THE ROOF STRUCTURAL SYSTEM, THE PERMANENT CROSS/LATERAL BRACING, BRACING TO TRANSFER MEMBER BUCKLING FORCES TO THE STRUCTURE AND ALL BRACING AND ANCHORAGE REQUIRED TO RESIST UPLIFT AND LATERAL FORCES. 1. FLAT AND FLOOR TRUGGEG MUST BE CLEARLY MARKED GO THAT THEY WILL BE INSTALLED RIGHT SIDE UP. THESE MARKS MUST REMAIN AFTER THE FLOORING, SHEATHING AND

INSULATION HAVE BEEN INSTALLED. THE INTENT OF THE ABOVE REQUIREMENTS IS TO PROVIDE ALL INFORMATION ON FRAMING, CONNECTIONS AND BRACING ON ONE COMPOSITE SET OF PLANS APPROVED BY THE ARCHITECT OR ENGINEER OF RECORD TO AID IN THE REVIEW, APPROVAL AND FIELD INSPECTIONS FOR THE PORTION OF THE PROPERTY.

BUILDER TO PROVIDE G3XDESIGN FINAL ENGINEERED TRUSS DRAWINGS SEAL BY TRUSS ENGINEER PRIOR TO CONSTRUCTION, FOUNDATION AND VERTICAL STRUCTURA SUPPORTS MAY CHANGE AS A RESULT OF FINAL TRUSS INGINEERING PROVIDED. REVISIONS SHALL BE COMMUNICATED IN WRITING

GXDESIGN, LLC RESERVES THE RIGHT TO MAKE SUBSTITUTIONS TO ANY CONNECTOR SPECIFIED AFTER SUBMITTAL OF FINAL SIGNED AND SEALED TRUSS DRAWINGS HAVE BEEN PROVIDED FOR REVIEW BUILDER SHALL COORDINATE WITH G3XDESIGN, LLC PRIOR TO CONSTRUCTION

SHOWN ON PERMIT PLANS, BUILDER IS RESPONSIBLE FOR ANY REVISIONS PRIOR TO FINAL TRUSS APPROVAL

FINAL APPROVED TRUSS DRAWINGS MAY REQUIRE ADDITIONAL FOUNDATION SUPPORTS, COLUMNS, AND BEAMS NOT

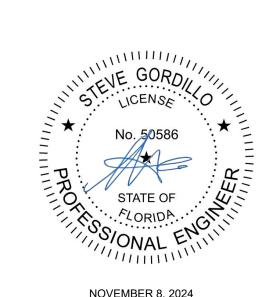
A permit issued shall be construed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a

period of six months after the time the work is commenced. ANY UNAUTHORIZED USE, REPRODUCTION OR DUPLICATION OF THESE DRAWINGS WITHOUT THE THE

EXPRESS WRITTEN CONSENT OF THE BUILDER,

DO NOT SCALE DIMENSIONS FOR CONSTRUCTION PURPOSES. IN THE EVENT THAT A DIMENSION IS UNCLEAR OR MISSING CONTACT THE ENGINEER IN

DESIGNER AND ENGINEER IS STRICTLY PROHIBITED



I CERTIFY THAT TO THE BEST OF THE ENGINEER'S KNOWLEDGE AND BELIEF ALL OF THE STRUCTURAL ELEMENTS AND SYSTEMS HAVE BEEN DESIGNED TO BE IN COMPLIANCE WITH THE 8TH EDITION OF THE 2023 RESIDENTIAL FLORIDA BUILDING CODE FOR BASIC WIND SPEED OF 150 MPH, EXPOSURE "D".

THE DRAWING IS SEALED FOR THE STRUCTURAL PORTIONS ONLY. ALL OTHER ELEMENTS, SYSTEMS AND ASSEMBLIES ARE THE RESPONSIBILITY OF THE BUILDER

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ENGINEERING DESIGN

G3X DESIGN, LLC 2237 CLIMBING IVY DR TAMPA, FL 33618 (813) 928-8339 FL C.A. #31107

NFI SFN

13222 3RD STREET EAST MADERIA BEACH, FL

BUILDER

OMAR ABBAS ABBAS DEVELOPMENT **BUILDING CONTRACTOR**

210 S PINELLAS AVE

SUITE 220

727-946-0475

DESIGNER

Curtis Morgan Morgancastle Studio, Inc.

9324 Wildwood Ave.

Residential Design Services

morgancastlestudio@gmail.com

Hudson, FL 34669

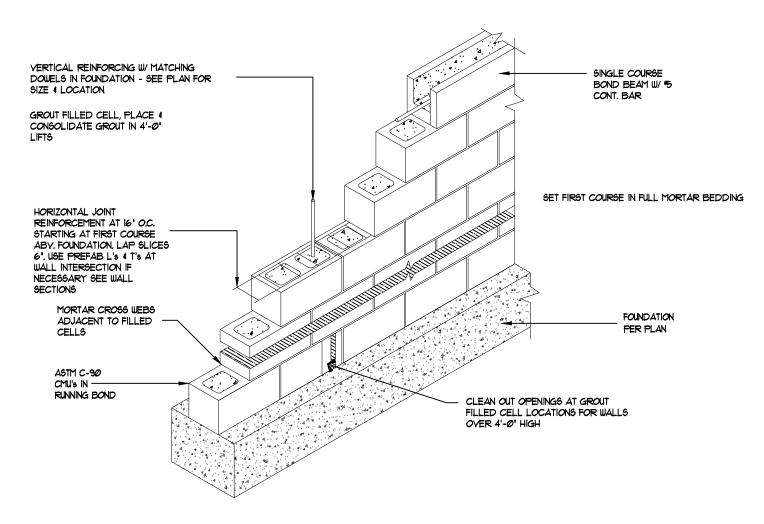
Phone: (727)247-8148

REVISIONS

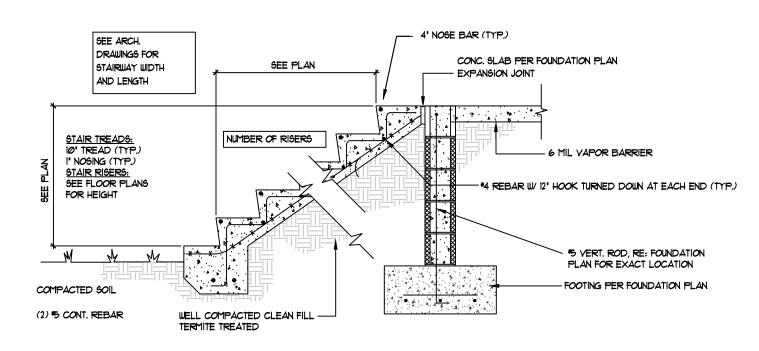
FINAL PERMIT

SET SHEET

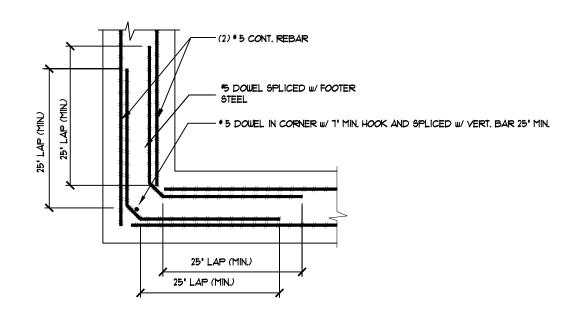
DATE 11-08-2024



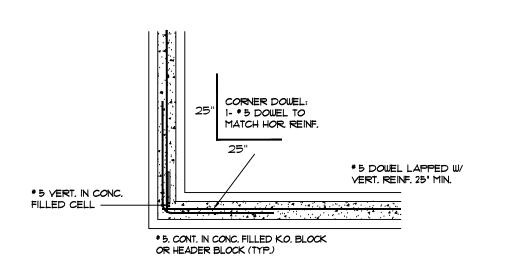
TYPICAL CMU REINFORCEMENT DETAIL



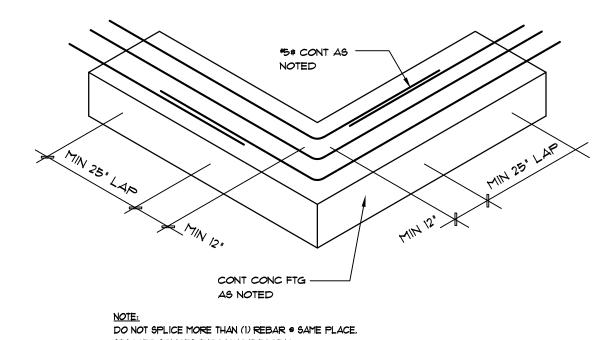
TYPICAL CONCRETE STEPS ON GRADE



FOOTER CORNER REINFORCEMENT DETAIL

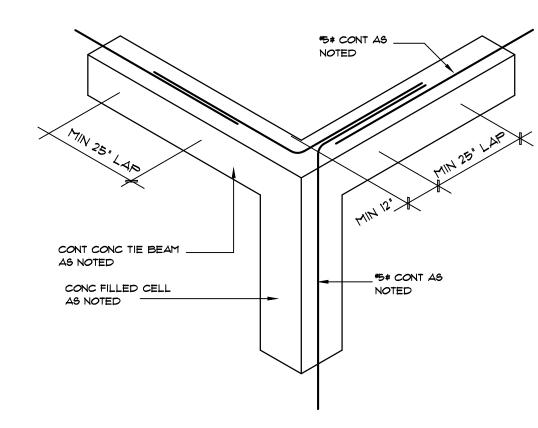


STEMWALL CORNER REINFORCEMENT DETAIL

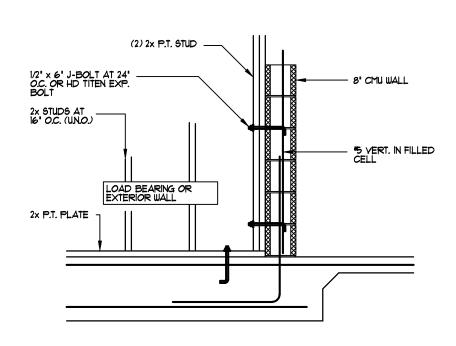


DETAIL (CONTINUITY OF FOOTING STEEL)

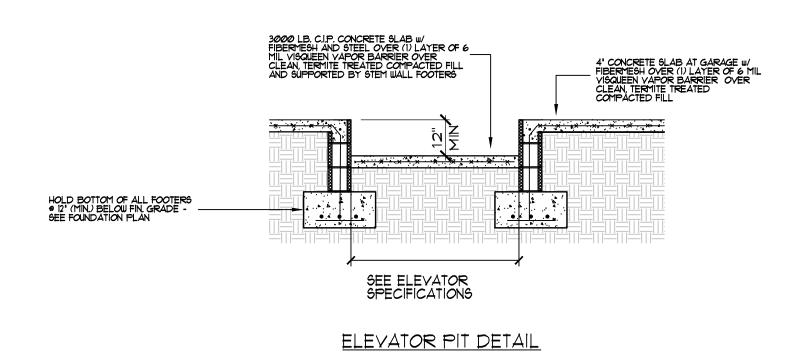
STAGGER SPLICES THROUGHOUT FOOTING



DETAIL (CONTINUITY OF BEAM STEEL)



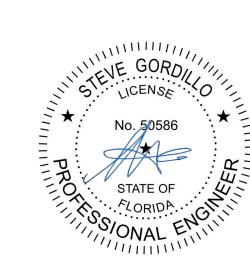
FRAME WALL TO MASONRY WALL DETAIL



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NOVEMBER 8, 2024

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ENGINEERING DESIGN

G3X DESIGN, LLC 2237 CLIMBING IVY DR TAMPA, FL 33618 (813) 928-8339 FL C.A. #31107



NELSEN CUSTOM RESIDENCE

13222 3RD STREET EAST MADERIA BEACH, FL

BUILDER

OMAR ABBAS

ABBAS DEVELOPMENT BUILDING CONTRACTOR 210 S PINELLAS AVE SUITE 220 727-946-0475

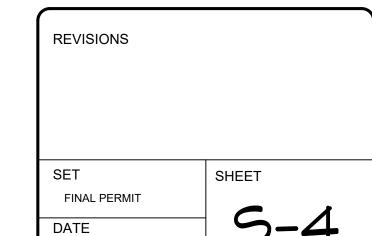
DESIGNER

Curtis Morgan

Morgancastle Studio, Inc. Residential Design Services

9324 Wildwood Ave. Hudson, FL 34669 Phone: (727)247-8148

morgancastlestudio@gmail.com



11-08-2024

STRUCTURAL WIND DESIGN CRITERIA

FLORIDA BUILDING CODE 2023 AND ASCE 7-22

AS DEFINED IN ASCET-22 THIS STRUCTURE MEETS THE REQUIREMENTS OF AN ENCLOSED STRUCTURE IN WIND DEBRIS REGION AND HAS BEEN DESIGNED WITH AN INTERNAL PRESSURE COEFFICIENT OF +Ø.18 AND -Ø.18.

COMPONENT AND CLADDING DESIGN PRESSURE SHOWN ABOVE ARE YUIT MUST BE CONVERTED TO ASD FOR PRODUCT APPROVAL PURPOSES

BASIC WIND SPEED	150 MPH
RISK FACTOR	11
EXPOSURE CATEGORY	D
WIND DIRECTIONALITY FACTOR	Ø.85
TOPOGRAPHIC FACTOR	1.0
GROUND ELEVATION FACTOR	1.0
VELOCITY PRESSURE EXPOSURE COEFFICIENT	Ø.575
INTERNAL PRESSURE COEFFICIENT	ENCLOSED +/- Ø.18
STRUCTURE HEIGHT	35'
BUILDING HEIGHT ADJUSTMENT FACTOR	1.05

FLORIDA PRODUCT APPROVAL SUBMITTALS

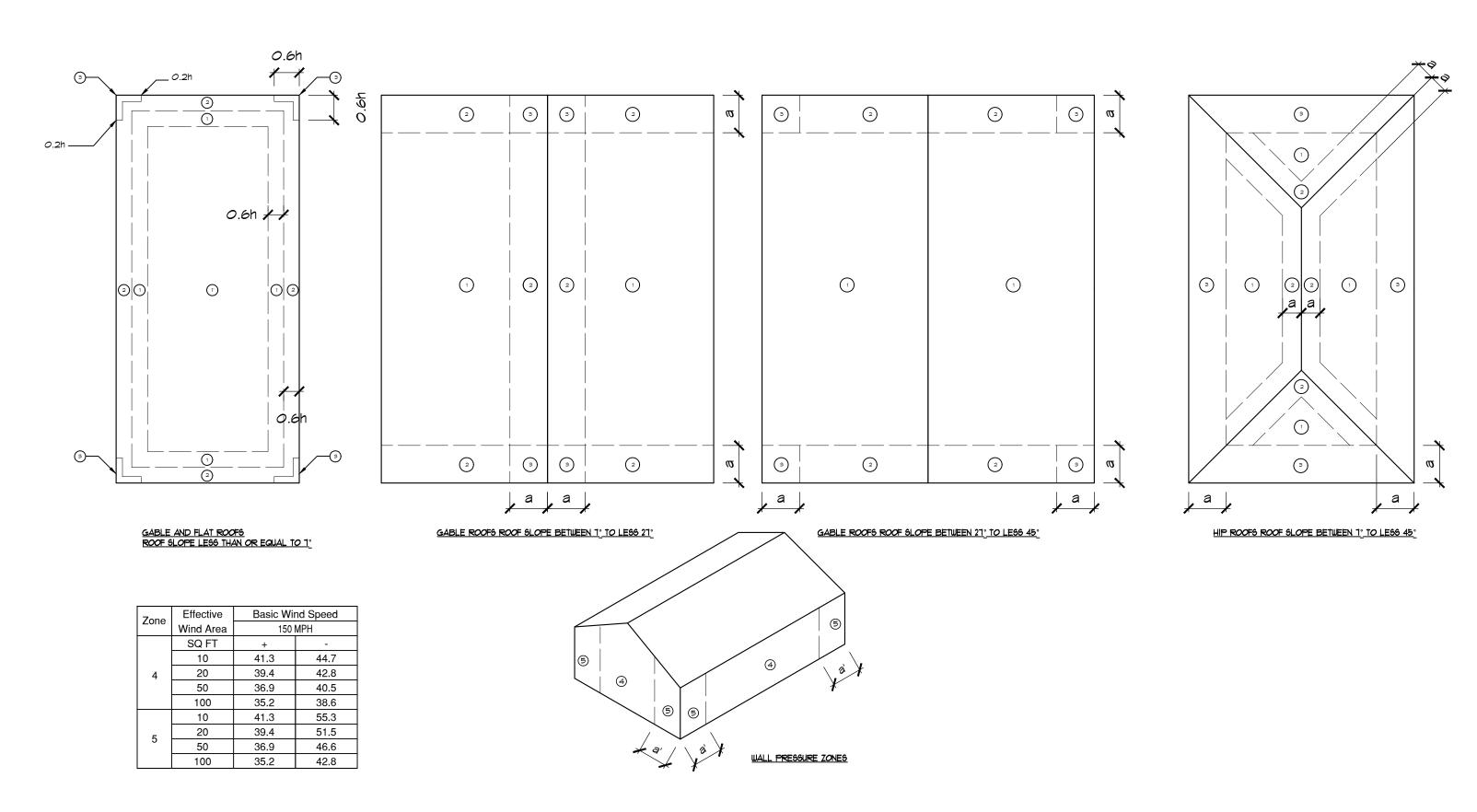
FLORIDA PRODUCT APPROVAL SHEETS SHALL BE SUBMITTED FOR THE MANUFACTURER UTILIZED FOR THE REQUIRED EXTERIOR PRODUCTS AND MEET REQUIRED DESIGN PRESSURES STATED FOR THE PROJECT

IT IS THE BUILDERS RESPONSIBILITY TO VERIFY THAT ALL MATERIALS AND INSTALLATION HAVE MET THE REQUIREMENT IN THE FLORIDA PRODUCT APPROVAL SHEETS.

FLORIDA PRODUCT APPROVALS LISTED BELOW ARE INFORMATIONAL AND CAN BE SUBSTITUTED FOR PRODUCTS THAT ARE EQUAL OR GREATER IN PERFORMANCE

ALL WINDOWS AND DOORS ABOVE DFE MUST BE IMPACT RESISTANT AND MEET A MINIMUM DESIGN PRESSURE OF +41.3/-55.3 PSF (ASD VALUE) EXPOSURE D @ 150 MPH FOR WALL ZONE 4 & 5

GARAGE DOORS SHALL MEET A MINIMUM DESIGN PRESSURE OF +38.8/-41.0 PSF

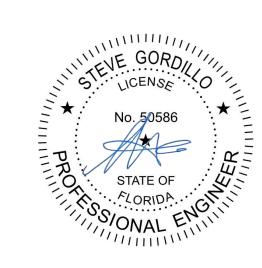


FLAT ROOF TO 1°					GABL	E 1° TO	2ذ	GABLE 20° TO 27°			GABLE 27° TO 45°				
DESIGN WIND PRESSURE (PSF)				DESIGN WIND PRESSURE (PSF)			DESIGN WIND PRESSURE (PSF)			DESIGN WIND PRESSURE (PSF)					
Zone	Effective	Basic Wi	ind Speed	Zone	Effective	Basic W	ind Speed	Zone	Effective	Basic Wi	nd Speed	Zone	Effective	Basic Wi	nd Speed
Zone	Wind Area	150	MPH	Zone	Wind Area	150	MPH		Wind Area	150	MPH	Zone	Wind Area	150	MPH
	SQ FT	+	-		SQ FT	+	-		SQ FT	+	-		SQ FT	+	-
	10	16.8	37.7		10	16.8	76.3		10	16.8	58.8		10	37.7	69.4
1'	20	15.8	37.7	1	20	15.8	65.6	1	20	15.8	53.0	1	20	34.5	58.8
	50	14.3	37.7	j !	50	14.3	51.5		50	14.3	45.6		50	30.3	44.9
	100	13.3	37.7		100	13.3	40.8		100	13.3	40.0		100	27.0	34.3
	10	16.8	65.8		10	16.8	100.8		10	16.8	93.8		10	37.7	76.3
	20	15.8	61.4	2	20	15.8	87.0	2	20	15.8	80.1	2	20	34.5	68.2
1	50	14.3	55.8		50	14.3	68.9		50	14.3	62.1		50	30.3	57.5
	100	13.3	51.3		100	13.3	55.1		100	13.3	48.3		100	27.0	49.3
	10	16.8	86.7		10	16.8	132.3		10	16.8	111.2		10	37.7	93.8
2	20	15.8	81.3		20	15.8	113.2	3	20	15.8	94.4		20	34.5	81.6
2	50	14.3	73.8	3	50	14.3	88.2		50	14.3	72.1	3	50	30.3	65.
	100	13.3	68.2		100	13.3	69.4		100	13.3	55.3		100	27.0	53.
3	10	16.8	118.3												
	20	15.8	107.1	HIP ROOF 7° TO 20°				HIP ROOF 20° TO 27°				HIP ROOF 27° TO 45°			
	50	14.3	92.3] '	"" 1		20								0 10
	100	13.3	81.3		DEGICAL WILLIAM PRESCRIPE (POE)				DECICAL WIND DECCLIPE (DCE) DECICAL WIND DECCLIF				F (DOF)		
				-	DESIGN WIND PRESSURE (PSF)			DESIGN WIND PRESSURE (PSF) Effective Basic Wind Speed			DESIGN WIND PRESSURE (PSI			, ,	
				Zone	Effective		ind Speed	Zone	Effective			Zone	Effective		
0±=0					Wind Area		MPH		Wind Area		MPH		Wind Area		MPH
OTES			_		SQ FT	+	-	4	SQ FT	+	-	4	SQ FT	+	-
	N PRESSURE				10	30.8	69.4	1	10	30.8	55.3	4	10	30.8	58.
						26.5	49.0	- ' 	20	26.5	50.				
(SUM OF EXTERNAL AND INTERNAL PRESSURES) APPLIED NORMAL TO ALL SURFACES. COMPONENT MANUFACTURER SHALL USE THE					50	21.1	50.5	1	50	21.1	40.6		50	21.1	39.
					100	16.8	42.3		100	16.8	34.3		100	16.8	30.
					10	30.8	90.3	1 .	10	30.8	76.3	┨ .	10	30.8	69.
				2	20	26.5	81.4	2	20	26.5	65.8	2	20	26.5	58.
HIGHER OF THE TWO NUMBERS FOR 100 16.8 6					69.5	4	50	21.1	51.9	4	50	21.1	44.9		
					60.7	-	100	16.8	41.3		100	16.8	34.		
AIT EICADEL GOGARE TOOTAGE.					10	30.8	97.2	4	10	30.8	76.3	4	10	30.8	90.
					20	26.5	87.6			26.5	65.8		20	26.5	75.
				_				١,	20			1 ,			
				3	50	21.1 16.8	79.7 64.9	3	20 50 100	21.1	51.9 41.3	3	50 100	21.1 16.8	56.1 41.3

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NOVEMBER 8, 2024

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2237 CLIMBING IVY DR TAMPA, FL 33618 (813) 928-8339 FL C.A. #31107



BUILDER

OMAR ABBAS ABBAS DEVELOPMENT BUILDING CONTRACTOR 210 S PINELLAS AVE SUITE 220

727-946-0475

MADERIA BEACH, FL

DESIGNER

Curtis Morgan Morgancastle Studio, Inc.

Morgancastle Studio, Inc. Residential Design Services

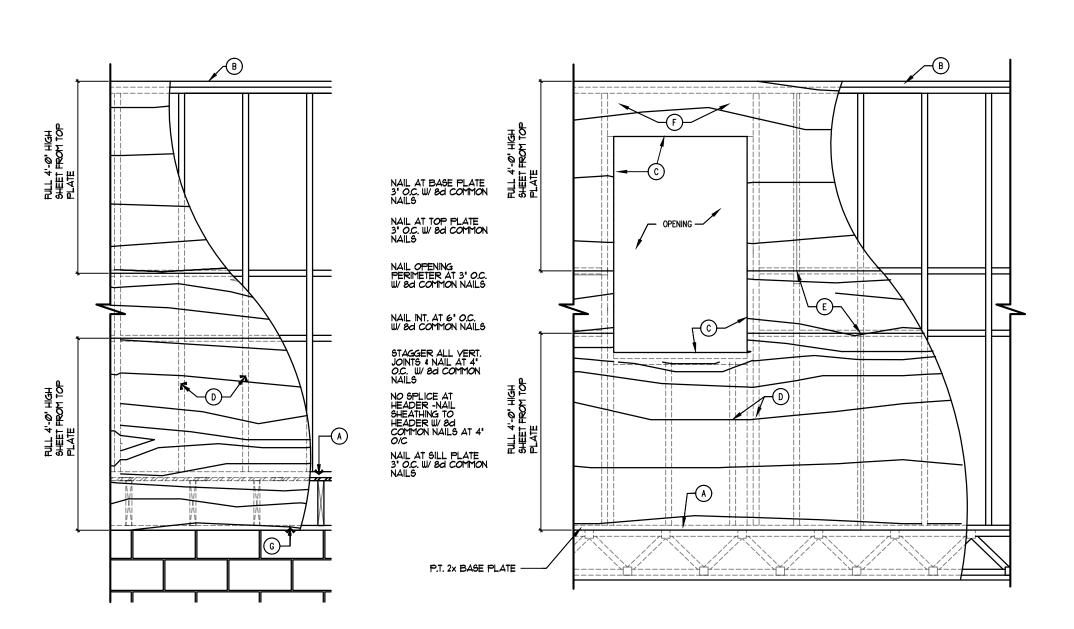
9324 Wildwood Ave. Hudson, FL 34669 Phone: (727)247-8148

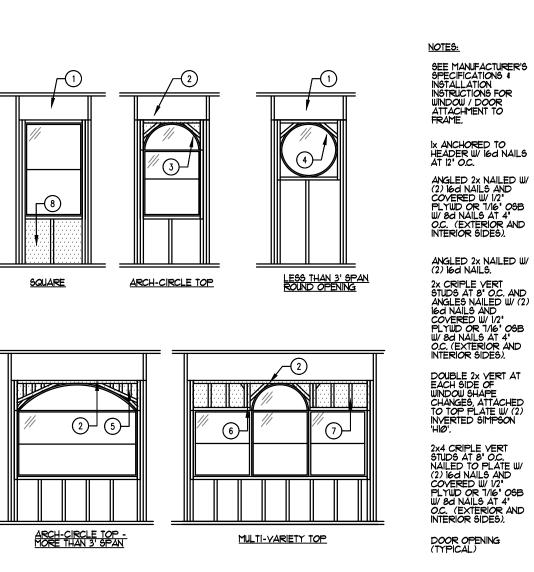
morgancastlestudio@gmail.com

REVISIONS

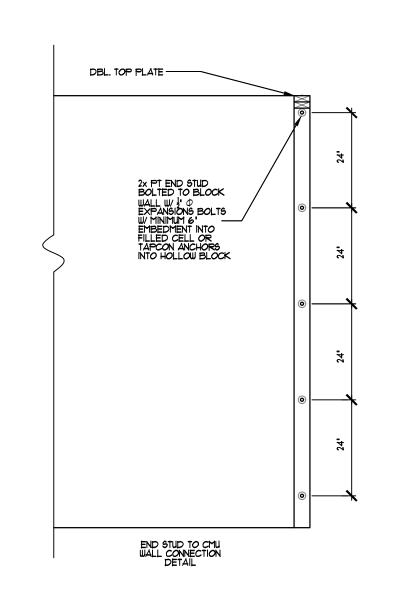
SET SHEET FINAL PERMIT

DATE 11-08-2024 5-5

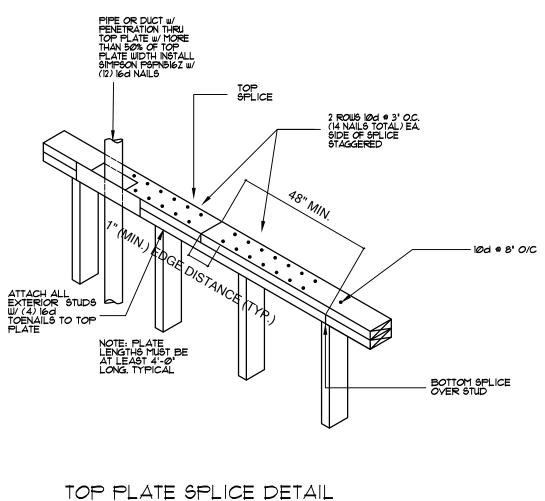




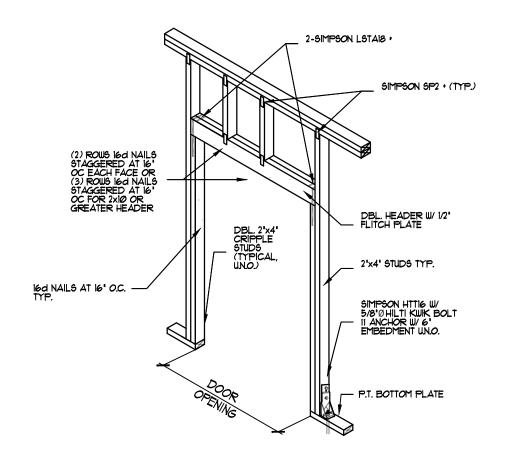
WINDOW & DOOR MOUNTING

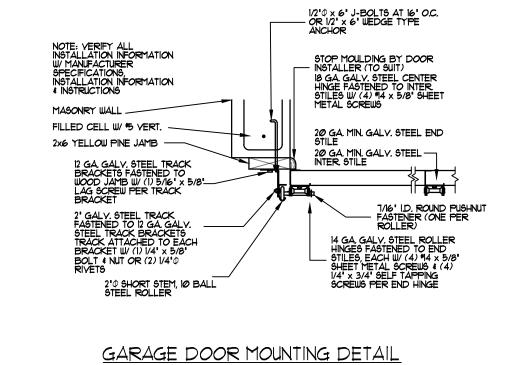


FRAME WALL TO CMU WALL DETAIL



WALL SHEATHING INSTALLATION & NAILING SCHEDULE





HTT / HDU TENSION TIE

2'x BUILT-UP COLUMN, SOLID MEMBER OR P.L. 1.8E Fb=2400 PSI (AS PER PLANS)

HTT4 w/ (18) lØd NAILS (36lØ* SPF)
 HTT5 w/ (26) lØd NAILS (461Ø* SPF)

• DTT2Z w/ (8) 1/4" x 1 1/2" SDS (1835* SPF) HDU2-SDS2.5 w/ (6) 1/4" x 2 1/2" SDS (2215* SPF)

HDU4-5D52.5 w/ (10) 1/4' x 2 1/2' 5D5 (3285* 5FF)
 HDU5-5D52.5 w/ (14) 1/4' x 2 1/2' 5D5 (4065* 5FF)

• HDU8-5D52.5 w/ (20) 1/4" x 2 1/2" 5D5 (5665* 5PF)

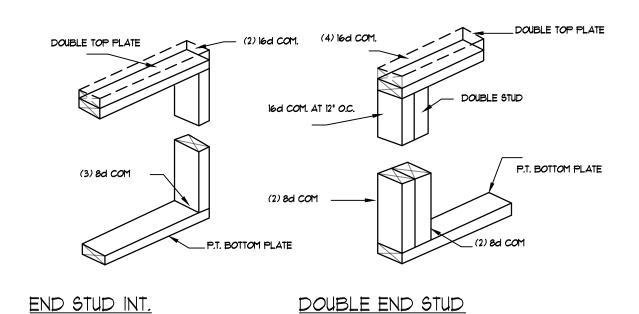
• HDUII-6D62.5 w/ (30) 1/4" x 2 1/2" SD6 (6865* SPF)

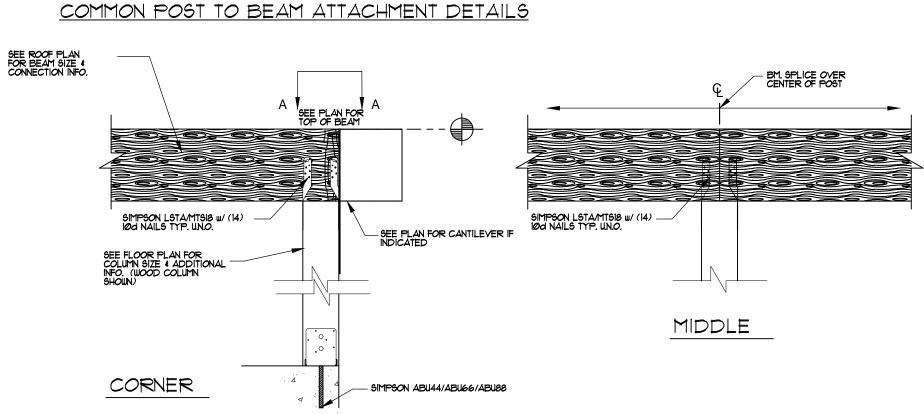
5/8' DIA x 12' EPOXIED ATR MIN 8' EMBEDMENT w/ WASHER 4 NUT
 FOR DT12Z USE 1/2' DIA x 12' EPOXIED ATR MIN 8'

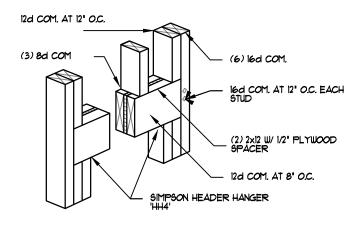
EMBEDMENT W/ WASHER & NUT

FOR HDUS USE 7/8" DIA. x 12" EPOXIED A.T.R. MIN. 8"
EMBEDMENT W/ WASHER & NUT

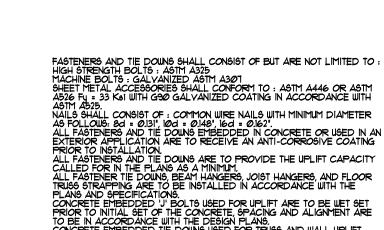
FOR HDUII USE 1" DIA. x 12" EPOXIED A.T.R. MIN. 8"
EMBEDMENT W/ WASHER & NUT







STUD SHOE



		USE (2) SIMPSON LSTAIS FRONT AND BACK © CORNERS USE 1-SIMPSON LSTAIS © EA. EXT. FACE.
ACITY		USE SIMPSON ABU

SIMPSON HEADER HANGER 1H14'	HIGH STRENGTH BOLTS: ASTM A325 MACHINE BOLTS: GALVANIZED ASTM A3Ø7 SHEET METAL ACCESSORIES SHALL CONFORM TO: ASTM A
EXT./BEARING WALL HEADER	A526 Fy = 33 Kai WITH G90 GALVANIZED COATING IN ACCO ASTM A525. NAILS SHALL CONSIST OF: COMMON WIRE NAILS WITH MINII AS FOLLOWS: 8d = 0.131", 10d = 0.148", 16d = 0.162". ALL FASTENERS AND TIE DOWNS EMBEDDED IN CONCRETI EXTERIOR APPLICATION ARE TO RECEIVE AN ANTI-CORR PRIOR TO INSTALLATION.
CONDUIT 1-1/2" OR GREATER	ALL FASTENERS AND TIE DOWNS ARE TO PROVIDE THE UF CALLED FOR IN THE PLANS AS A MINIMUM. ALL FASTENER TIE DOWNS, BEAM HANGERS, JOIST HANGER TRUSS STRAPPING ARE TO BE INSTALLED IN ACCORDANC PLANS AND SPECIFICATIONS. CONCRETE EMBEDDED 'J' BOLTS USED FOR UPLIFT ARE T PRIOR TO INITIAL SET OF THE CONCRETE, SPACING AND A TO BE IN ACCORDANCE WITH THE DESIGN PLANS.
	CONCRETE EMBEDDED TIE DOWNS USED FOR TRUSS AND ARE TO BE PLACED AROUND EMBEDDED REINFORCING F PLACING GROUT.

MODEL	COLUMN	ANCHOR	LOAD CAPACITY
ABU44	4x4 P06T	5/8 ° ∮ BOLT	2140 UP/6665 DOWN
ABU66	6x6 P09T	5/8 ° ∮ B <i>O</i> LT	2300 UP/12,000 DOWN
ABU88	8x8 P0\$T	(2) 5/8 ' Ø BOLT	2320 UP/24,335 DOWN

ND WALL UPLIFT G PRIOR TO CONNECTOR MATCH COLUMN SIZE

 4'-9'
 (2)
 (3)
 (2)
 (2)
 (1) L5TA3Ø

 9'-16'
 (3)
 (3)
 (2)
 (3)
 (2) L5TA3Ø

HEADER DETAIL (UPLIFT CONNECTIONS)

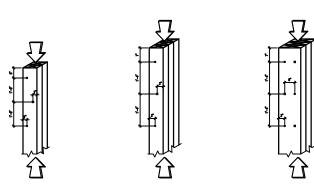
|-4' (|) (2) (|) (2) (|) L6TA3Ø

HEADER SUPPORT NO. OF JACKS & STUDS

REQ. AT OPENINGS

 OPENING SIZE
 2' x 4' WALL
 2' x 6' WALL

 JACKS STUDS EA END EA END EA END
 STUDS EA END



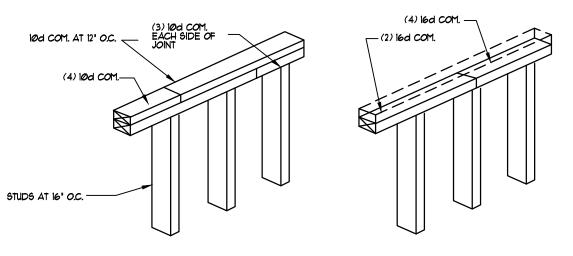


I. ADJACENT NAILS ARE DRIVEN FROM OPPOSITE SIDES OF THE COLUMN

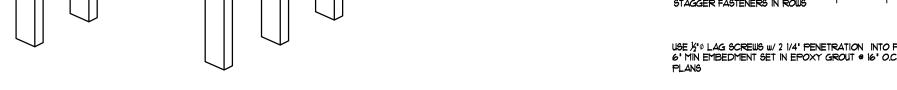
- 2. ALL NAILS PENETRATE AT LEAST 3/4 OF THE THICKNESS OF THE LAST LAMINATION 3. EACH 300d COMMON NAIL MAY BE REPLACED W (2) 160d COMMON NAILS. (ONE INTO EACH OUTSIDE FACE OF BLLC., SAME NUMBER OF ROWS, SAME SPACING)
- 4. FOR 4-PLY, PROVIDE 1/4" DIA. x 5 1/2" LAG SCREWS OR EQUAL (SPACE AS SHOWN FOR 3-PLY) 5. FOR 5-PLY, PROVIDE 1/4" DIA. x 1" LAG SCREWS OR EQUAL (SPACE AS SHOWN FOR 3-PLY)

6. REFER TO NDS SECTION 15.3 FOR ADDITIONAL INFORMATION

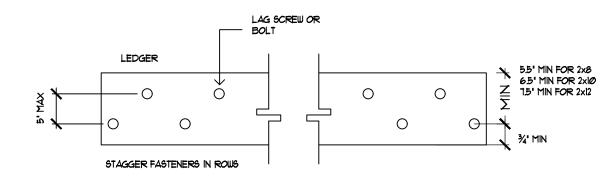
TYPICAL FRAME HEADER (LOAD BEARING WALL)				
ROUGH OPENING	2x4 FRAME WALL	2x6 FRAME WALL		
UP TO 4'	MIN (2) 2x12 W/ ½" PLYWOOD FLITCH	MIN (3) 2x12 W/½" PLYWOOD FLITCH		
4'-0" TO 6'-0"	MIN (2) 2x12 W/ ½" PLYWOOD FLITCH	MIN (3) 2x12 W/1/2" PLYWOOD FLITCH		
6'-0' TO 8'-0'	MIN (2) 2x12 W/ ½" PLYWOOD FLITCH	MIN (3) 2x12 W/½" PLYWOOD FLITCH		
OVER 8'-0"	MIN 2 PC6 OF 1 3/4" x 11 1/4" LVL BEAM	MIN 3 PCS OF 1 3/4" x 11 1/4" LVL BEAM		



TOP PLATE FASTENERS





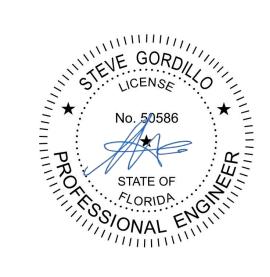


USE ½' $^{\circ}$ LAG SCREWS w/ 2 1/4' PENETRATION INTO FRAMING MEMBER OR ½' $^{\circ}$ BOLTS W/ 6' MI EMBEDMENT SET IN EPOXY GROUT $^{\circ}$ 16' O.C. UNLESS SPECIFIED OTHERWISE ON

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G3X DESIGN, LLC 2237 CLIMBING IVY DR TAMPA, FL 33618 (813) 928-8339 FL C.A. #31107



13222 3RD STREET EAST MADERIA BEACH, FL

BUILDER

OMAR ABBAS ABBAS DEVELOPMENT **BUILDING CONTRACTOR** 210 S PINELLAS AVE SUITE 220

727-946-0475

DESIGNER

Curtis Morgan

Morgancastle Studio, Inc. Residential Design Services

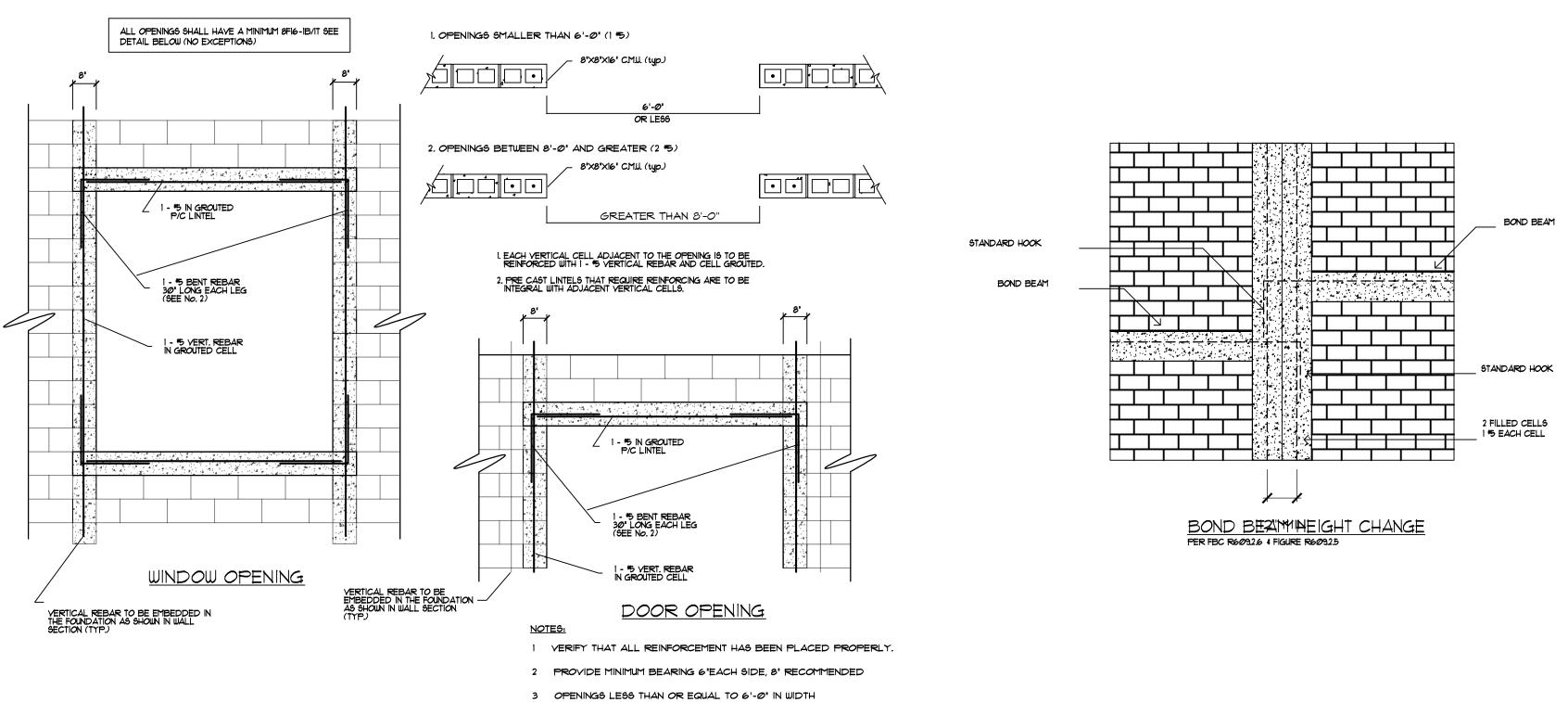
> 9324 Wildwood Ave. Hudson, FL 34669 Phone: (727)247-8148

morgancastlestudio@gmail.com

REVISIONS

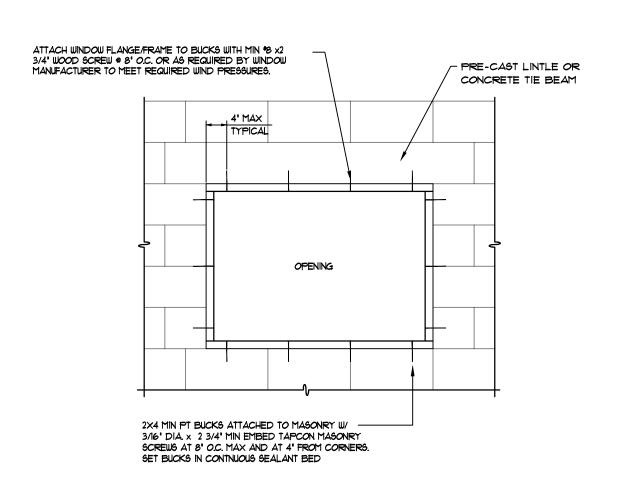
SET SHEET FINAL PERMIT

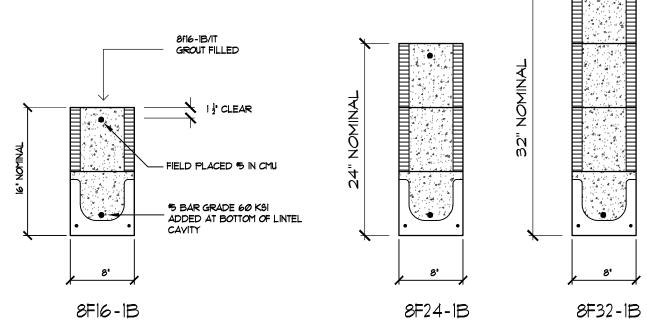
DATE 11-08-2024

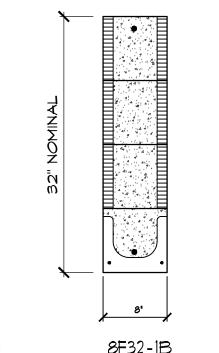


MASONRY OPENING DETAILS

DO NOT REQUIRE ADDITIONAL STEEL REINFORCEMENT UNLESS NOTED ON FLOOR PLAN.







 All values based on minimum 4 inch nominal bearing.
 Exception: Safe loads for unfilled lintels must be reduced by 20 % if bearing length is less than 6-1/2 inches.
 NR = Not Rated. Safe loads are superimposed allowable load.
 Safe loads based on Grade 40 or Grade 60 field rebar. 4. Sate loads based on Grade 40 of Grade 60 fletch repart.

5. Additional lateral load capacity can be obtained by the designer by providing additional reinforced masomy above the precast lintel. See Reinforced CMU on Page 4.

6. One *T rebar may be substituted for two *5 rebars in 8' lintels only.

1. The designer may evalute concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away.

ENGINEERING SPECIFICATIONS

PRODUCT DESCRIPTION High strength precast concrete lintels designed to be unfilled to form a composite reinforced beam using concrete masonry units. MATERIALS

• f'c 8' precast lintels = 3500 psi • f'c 8' prestressed, 6' and 12' precast lintels = 6000 psi • f'c 4' precast lintels = 3000 psi

Grout per ASTM C476 f'g = 3000 psi w/ maximum 3/8 inch aggregate and 8 to 11 inch slump.

1. Provide full mortar head and bed joints. Shore filled lintels as required. . Installation of lintel must comply with architectural and/or structural orawings.
4. U-Lintels are manufactured with 5-1/2 inch long notches at

ends to accomodate vertical cell reinforcing and grouting.

5. Reference the CAST-CRETE Load Deflection Graph Brochure for lintel deflection information. 6. Bottom field added rebar to be located at the bottom of lintel cavity.

1. 1/32 inch diameter wire stirrups are welded to the bottom

SAFE LOAD TABLE NOTES

 Concrete masonry units (CMU) per ASTM C90 with minimum net area compressive strenght = 1900 psi • Rebar per ASTM A615 Grade 60 Prestressing strand per ASTM A416 Grade 270 low relaxation
 1/32 inch wire per ASTM A510
 Mortar per ASTM C270 Type M or 5

8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masorry units.

9. Safe load ratings based on rational design analysis per ACI 318 and ACI 530 and 03-0605.04. Florida Certificate of Product Approval number FLI58. I. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C926 or other approved coating.

I. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loaded should be checked for the combined loading with the

> Applied vertical load Applied horizontal load Safe vertical load + Safe horizontal load ≤ 1.0

8. For composite lintel heights not shown, use safe load from next lower height.

9. For lintel lengths not shown, use safe load from

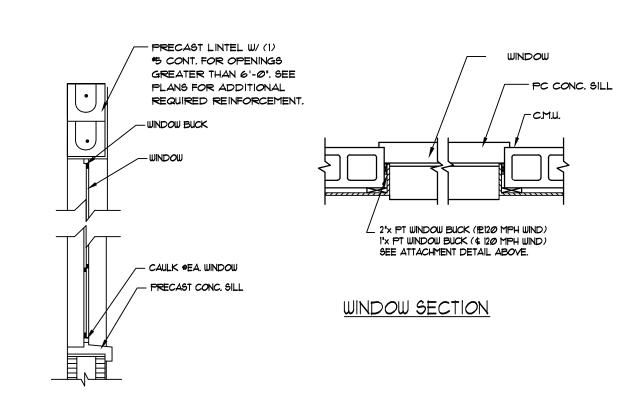
next longest length.

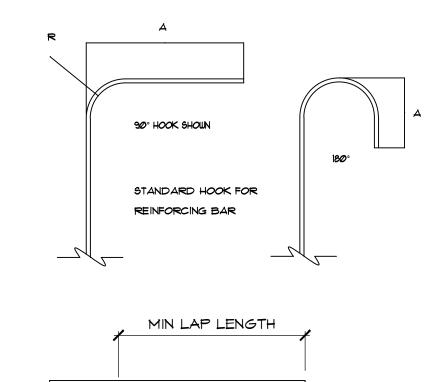
10 All safe loads in units of pounds per linear foot.

11. All safe loads based on simply supported span.

12. The number in the parenthesis indicates the percent reduction for grade 40 field added rebar. Example: 1'-6' lintel Type 8F32-1B safe gravity load = 6412 (15) w/ 15% reduction =12 6412 (.85) = 5501 plf

WINDOW ATTACHMENT TO CMU





LAP SPLICE

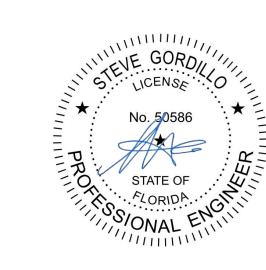
RECOMI	RECOMMENED END HOOKS AND LAP LENGTHS						
BAR	180? HOOK	307 HOOK	HOOK	LAP			
SIZE	А	A	Ŋ	Lb			
#3	5'	6'	1 1/4"	18'			
*4	6"	8'	1-1/2"	24"			
#5	יד	10"	2"	3 © '			
*6	ළ'	12"	2-1/4"	36'			
* T	10'	14"	2-3/4"	48'			
* 8	111	16'	3'	55'			
* 9	15'	19"	4-3/4"	62'			
*10	ידו"	22"	5-1/2"	69'			

STEEL LAP AND BEND

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ENGINEERING <u>DESIGN</u>

G3X DESIGN, LLC 2237 CLIMBING IVY DR TAMPA, FL 33618 (813) 928-8339



NELSEN **CUSTOM** RESIDENCE

13222 3RD STREET EAST MADERIA BEACH, FL

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DATE 11-08-2024

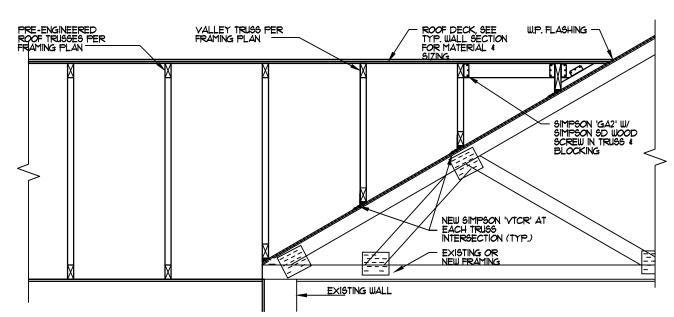
Curtis Morgan

Morgancastle Studio, Inc. Residential Design Services

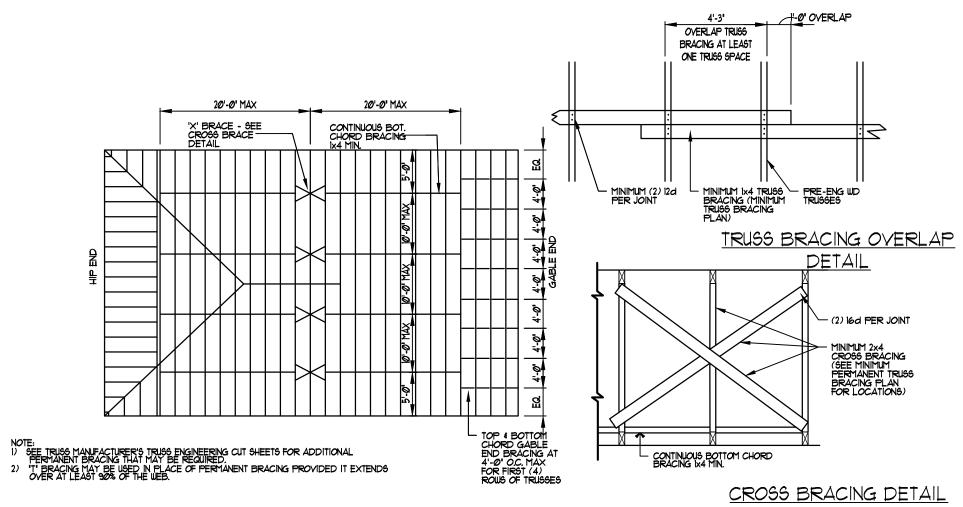
9324 Wildwood Ave. Hudson, FL 34669 Phone: (727)247-8148

morgancastlestudio@gmail.com

REVISIONS SET SHEET FINAL PERMIT



PRE=ENGINEERED VALLEY TRUSSES OVER EXISTING/NEW ROOF DETAIL (W/ SIMPSON VTCR & NO 2x CLEAT/NAILER)



MINIMUM PERMANENT TRUSS BRACING PLAN

DASHED LINES INDICATE TRUSSES OVERHANG NOTE: USE IØd RING SHANK NAILS 4'-Ø' END ZONE A'-Ø' END ZONE

ROOF NAILING SCHEDULE: NAILING ZONES (SHINGLE AND TILE)

ZONE 1: 10d RING SHANK NAILS @ 6'
O.C. ON EDGE AND 6' O.C. IN FIELD

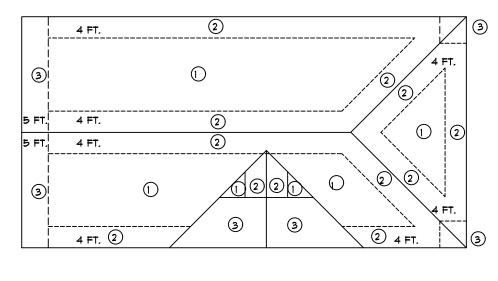
ZONE 2: 10d RING SHANK NAILS @ 6'
O.C. ON EDGE AND 6' O.C. IN FIELD

ZONE 3: 10d RING SHANK NAILS @ 4'
O.C. ON EDGE AND 4' O.C. IN FIELD

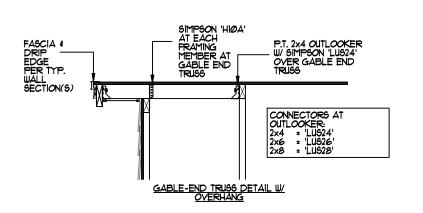


SHEATHING MAY BE INSTALLED VERTICALLY OR HORIZONTALLY, ATTACH PER NAILING SCHEDULE. PANEL EDGES WILL NEED TO BE ATTACHED TO STUD AND OR BLOCKING AT ALL EDGES.

A MINIMUM 1/8" SPACE IS RECOMMENDED BETWEEN PANELS AT EDGES AND END JOINTS TO ALLOW FOR EXPANSIONS, FASTENERS SHALL NOT PENETRATE SURFACE MORE THAN 1/8"



ROOF NAILING ZONE DIAGRAM



- WOOD TRUSS / JOIST

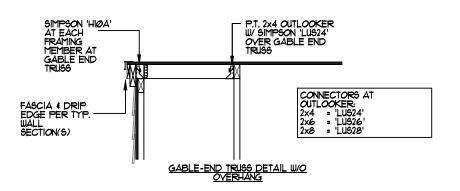
- 5/8' DIA. EPOXY BOLT W/

MIN. 5' EMBEDMENT

Simpson MGT Scale: N.T.S.

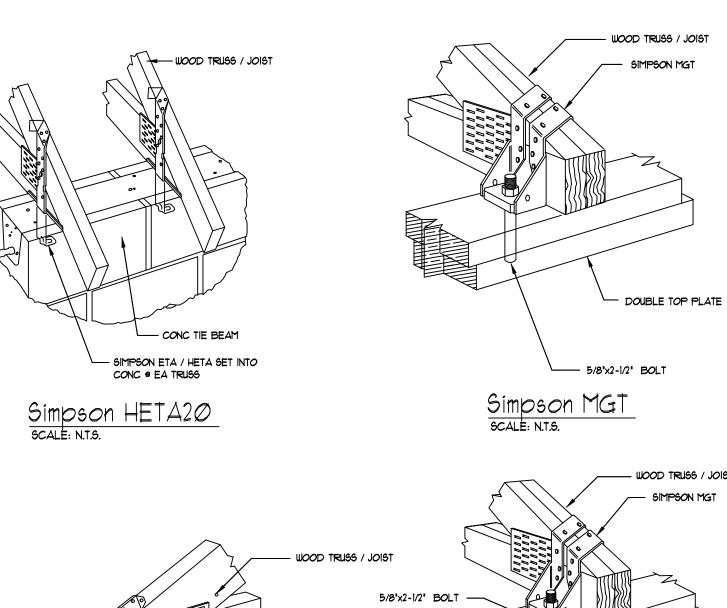
CONNECTOR SCHEDULE

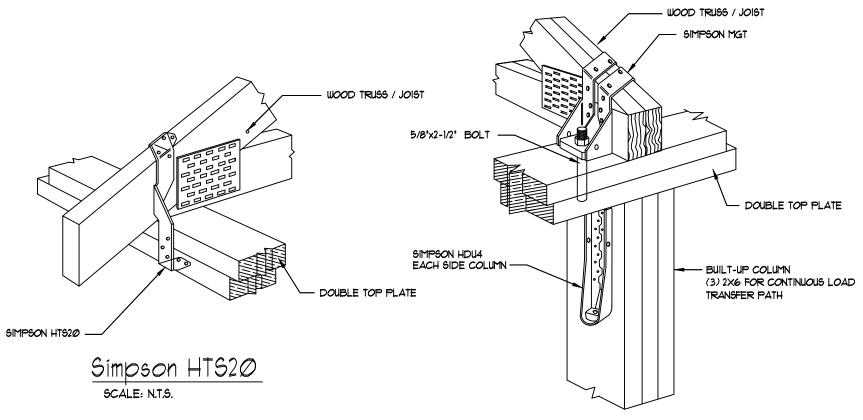
SIMPSON STRONG TIE



GABLE END ROOF DETAIL (SIMPSON CONNECTORS)

COMMON SIMPSON UPLIFT CONNECTORS

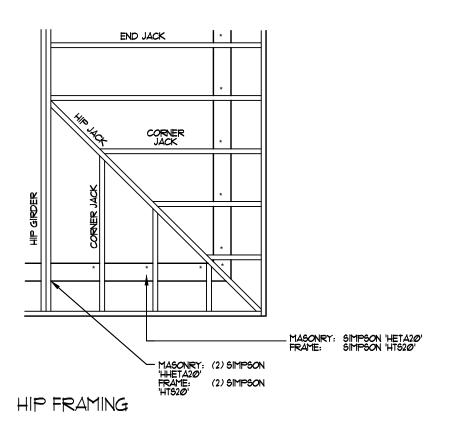


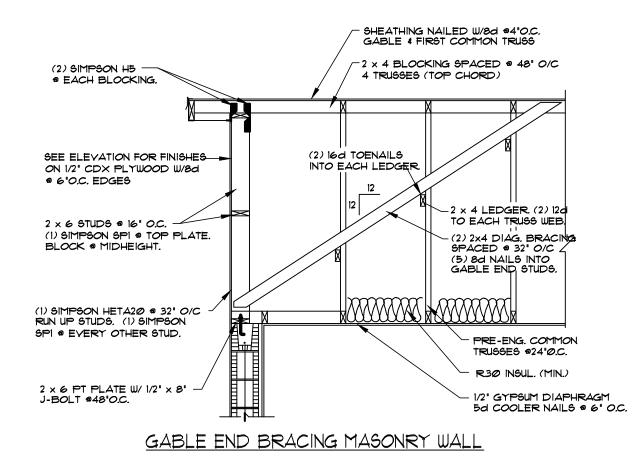


AT GIRDER TRUSS

SCALE: N.T.S.

FASTENERS CONNECTOR UPLIFT LOAD 3965 22-10d-1-5/8" MGT 3965 1-5/8" HETA2Ø 1-PLY 2Ø35 10-10d-1-1/2" BUILT-UP COLUMN
(3) 2X6 FOR CONTINUOUS LOAD HETA2Ø 2-PLY 2035 12-16d HTS2Ø 1175 10-10d TRUSS HTS2Ø 4-1/4x2-1/4 TITEN-CMU 1175 MSTAM24 9-10d MSTAM24 5-1/4x2-1/4 TITEN-CMU MSTAM36 13-1Ød MSTAM36 8-1/4x2-1/4 TITEN-CMU BUILT-UP COLUMNS





SEE STRUCTURAL NOTES AND NAILING
SCHEDULE FOR SHEATHING DETAIL

TOENAIL W/ (3) I6d NAILS

2 x 4 OUTLOOKER © 24' O/C

TOENAIL W/ (3) I6d NAILS

2 x 6 CONT. STUD © I6' O.C.
W/ (1) SIMPSON
SP2 © TOP 4 (1) SPI © BASE
© EACH STUD.

2 x 4 PANEL BLOCKING © 48' O/C
FOR THE FIRST (4) ROWS OF TRUSSES
FROM EA, END TYP. TOP 4 BOTTOM
ATTACHED W/ (3) I6-d TOENAILS

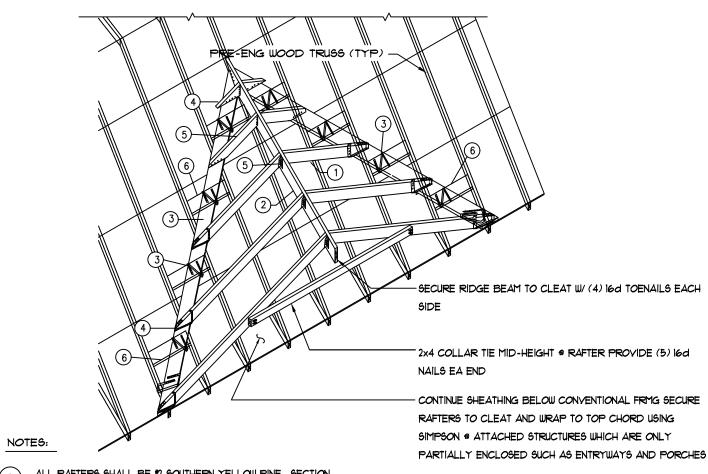
BLOCKING © CEILING.

GABLE END BRACING FRAME WALL

-STUD WALL (SEE TYP. SEC.)

CONVENTIONAL VALLEY FRAME

1/2" GYP W.B. DIAPH W. 5d COOLER NAILS @ 6" O.C.



- ALL RAFTERS SHALL BE *2 SOUTHERN YELLOW PINE. SECTION SIZE SHALL BE DETERMINED ACCORDING TO THE LENGTH OF SPAN AS FOLLOWS:
 - UP TO 8'-0'
 2x6

 8'-0' TO 12'-0'
 2x8

 12'-0' TO 15'-0'
 2x10

 15'-0' TO 18'-0'
 2x12
- 2 RIDGE BEAM SHALL BE *2 SOUTHERN YELLOW PINE. SECTION SIZE SHALL BE 2" (NOMINAL) LARGER THAN RAFTERS W/ A MAXIMUM RIDGE SIZE OF 2xi2 FOR 2xi2 RAFTERS.
- CLEAT SHALL BE A *2 SOUTHERN YELLOW PINE 2x10. IT SHALL
 BE FASTENED AS SHOWN W/ SIMPSON MSTA24 TENSION STRAPS

 BEACH TRUSS USING (14) ID ANAILS. INSTALL THE STRAP BY
 CUTTING A HOLE INTO THE SHEATHING BEACH SIDE OF TRUSS
 AND THREADING THE U-SHAPED STRAP FROM BOTTOM OF
 TRUSS TOP CHORD TO TOP OF CLEAT.
- SECURE RAFTERS TO RIDGE BEAM USING SIMPSON L95U2IØ

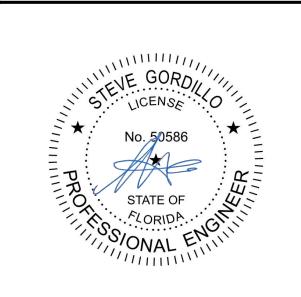
 SLOPING NAILS INTO 2x12 RAFTERS. USE (8) IØd & (6) IØd
 RESPECTIVELY FOR HT52Ø W/ (20) IØd NAILS FOR 2x12
 RAFTERS, (I6) IØd NAILS FOR 2x1Ø RAFTERS, (I4) IØd NAILS
 FOR 2x6 RAFTERS WITH SPANS OF 4'-Ø' OR GREATER 2x6
 RAFTERS W/ SPANS LESS THAN 4'-Ø' WITH SPANS LESS THAN
 4'-Ø' MAY BE TOE-NAILED TO RIDGE BEAM SHALL BE
 TOE-NAILED TO CLEAT W/ (8) IØd NAILS.
- WHERE NEEDED, PROVIDE FLAT 2X6 BLOCKING BETWEEN TRUSSES AT HANGER W/ (10) 10d NAILS INTO RIDGE BEAM AND (1) 10d x 1 1/2" SECURE BLOCKING W/ (3) 16d TOENAILS AT EACH END INTO TOP CORD. 2X10 RAFTERS, USE LSSU28 W/ (10) 10d & (5) 10d x 1 1/2" FOR 2X8 RAFTERS, USE LSU26 W/ (6) 10d & (5) 10d x 1 1/2" FOR 2X6 RAFTERS WITH SPANS OF 4"-0" OR GREATER. 2X6 RAFTERS W/ (8) 10d NAILS.
- (6) TOP CORD FOR INSTALLATION SUPPORT OF MSTA24 STRAPS.

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DO NOT SCALE DIMENSIONS FOR CONSTRUCTION PURPOSES. IN THE EVENT THAT A DIMENSION IS UNCLEAR OR MISSING CONTACT THE ENGINEER IN WRITING



NOVEMBER 8, 2024

I CERTIFY THAT TO THE BEST OF THE ENGINEER'S KNOWLEDGE AND BELIEF ALL OF THE STRUCTURAL ELEMENTS AND SYSTEMS HAVE BEEN DESIGNED TO BE IN COMPLIANCE WITH THE 8TH EDITION OF THE 2023 RESIDENTIAL FLORIDA BUILDING CODE FOR BASIC WIND SPEED OF 150 MPH, EXPOSURE "D".

THE DRAWING IS SEALED FOR THE STRUCTURAL PORTIONS ONLY. ALL OTHER ELEMENTS, SYSTEMS AND ASSEMBLIES ARE THE RESPONSIBILITY OF THE BUILDER

THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY STEVE GORDILLO, PE USING A DIGITAL SIGNATURE AND DATE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE DIGITAL SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES



NELSEN CUSTOM RESIDENCE

13222 3RD STREET EAST MADERIA BEACH, FL

BUILDER

OMAR ABBAS

ABBAS DEVELOPMENT
BUILDING CONTRACTOR

BUILDING CONTRACTO 210 S PINELLAS AVE SUITE 220 727-946-0475

DESIGNER

Curtis Morgan

Morgancastle Studio, Inc.
Residential Design Services

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morgancastlestudio@gmail.com

REVISIONS	
SET	SHEET
FINAL PERMIT DATE	G a